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A case from Dead Sea earthquakes**

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## Soft sediment deformation by Kelvin Helmholtz Instability: A case from Dead Sea earthquakes

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### Abstract

The standard explanation for soft sediment deformation is associated with overturn of inverted density gradients. However, in many cases, observations do not support this interpretation. Here we suggest an alternative in which stably stratified layers undergo a shear instability during relative sliding via the Kelvin–Helmholtz Instability (KHI) mechanism, triggered by earthquake shaking. Dead Sea sediments have long stood out as a classical and photogenic example for recumbent folding of soft sediment. These billow-like folds are strikingly similar to KHI structures and have been convincingly tied to earthquakes. Our analysis suggests a threshold for ground acceleration increasing with the thickness of the folded layers. The maximum thickness of folded layers (order of decimeters) corresponds to ground accelerations of up to 1 g. Such an acceleration occurs during large earthquakes, recurring in the Dead Sea.

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### 1. Introduction

The ubiquitous stratification in low-energy deposits, where density typically increases with depth, inhibits gravitational instabilities of the Rayleigh–Taylor type. Yet such deposits commonly show structural evidence of mechanical instabilities experienced in the unconsolidated state. Layer-parallel displace-

ments, not uncommon in soft sediments, force shear between layers and possibly drives instabilities of the Kelvin–Helmholtz (KH) type [1]. Layer-parallel shear in post-depositional situations can be driven by a number of mechanisms such as sloping substrates or water flow above the sediments. Yet, soft sediment deformations are observed also on vanishing slopes and at calm water environments. Sediments in the Dead Sea basin provide long environmental records comprising finely laminated layers, radiometrically dated to a precision of tens to hundreds years [2]. Laminated lake deposits, such as in the Quaternary

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41 Dead Sea, provide spectacular examples for such  
 42 deformation structures (Fig. 1) [2]. These structures  
 43 have been tied to strong earthquakes [3–8], providing  
 44 a source for shear energy. Earthquakes may leave  
 45 several types of marks on soft laminated beds, includ-  
 46 ing faulting, folding and fragmentation. Counting  
 47 laminae (thought to represent seasonal deposition)  
 48 provides a resolution approaching annual that recently  
 49 enabled matching of particular deformed laminae to  
 50 historically documented earthquakes [3].

51 The folding of soft sediments appears at various  
 52 magnitudes, seemingly indicating various stages of  
 53 the deformation. Folding can evolve from a wavy

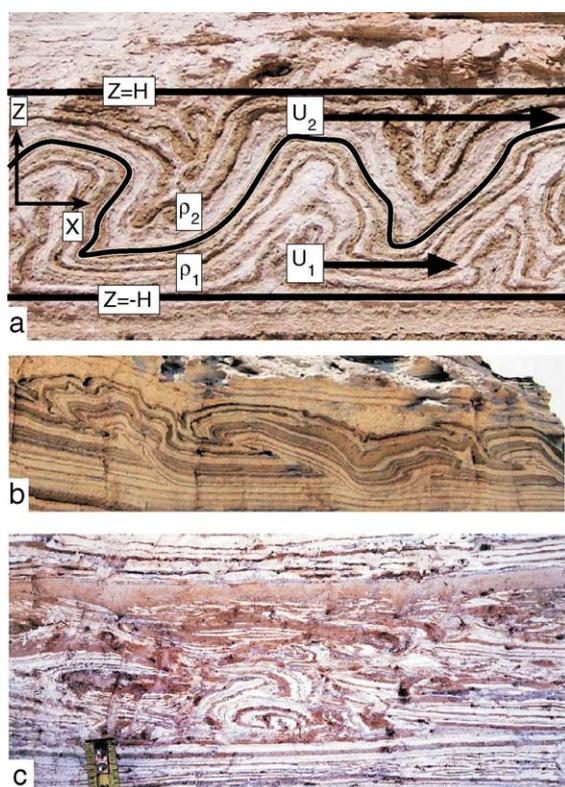


Fig. 1. Examples of different geometry of sediment foldings: (1a) linear wavy geometry, (1b) coherent billow vortices, and (1c) turbulent mixed breccia layer (Photos were taken from the Dead Sea region). In (1a) the speculated original condition supporting KHI is illustrated schematically: Two layers, of thickness  $H$ , initially horizontal and stably stratified ( $\rho_1 > \rho_2$ ), experience an earthquake shaking in the  $x$  direction. In response to the shaking, the denser lower layer moves more slowly than the upper one, forming shear at the interface. The interface, located initially at  $z=0$ , was perturbed becoming unstable with a wavy shape.

shape (Fig. 1a) which can be distorted further to a  
 billow-like or recumbent form (Fig. 1b). The layer  
 may deform further and become fully turbulent,  
 creating a thoroughly mixed breccia layer featuring  
 fragments from the original laminae (Fig. 1c)  
 [3,5,6].

Here we examine the feasibility for a KHI mechanism by which folds are being formed. The shear in the present context is one of fluid flow, not related to the elastic cyclic shear loading prior to liquefaction [9]. KHI was examined in numerous laboratory experiments [10] and numerical simulations [11]. The mechanism involves two horizontal layers that are stably stratified (the light layer overlies the heavy one, Fig. 1a). The layers move horizontally in the same direction but with different speeds, creating shear in the layers' interface. Such shear tends to rotate the beds, giving rise to an instability that uplifts the heavy layer above the lighter one (Fig. 1a). As a result, a wavy structure of billow vortices, distorted by the shear, is formed; the heavier lifted layer tends to collapse into the lighter layer and mix with it (Fig. 1b). If the shear persists, a mixed turbulent boundary layer is developed at the interface where the local shear within the billows forms secondary unstable vortices (note the small scale wiggles in Fig. 1b). These vortices cascade energy into smaller scales and promote the mixing [1] (Fig. 1c).

We attempt to construct a simple physical model to capture the dynamics of the phenomenon (Section 2) and examine its potential instability (Section 3). Finally, we discuss the applicability of the model to Dead Sea deposits (Section 4).

## 2. A simple model of sediment KHI instability

The folds amount to evidence that deformation took place while the sediment was in a state of unconsolidated mud, reasonably treated as a fluid. During the processes of sedimentation and loss of fluid, the suspension can be viewed as an array of particles falling through the suspending fluid at a steady state velocity. The sedimentation velocity decreases with increasing mass fraction  $\chi$ . Gradients in  $\chi$  tend to form sharp fronts between layers of uniform density, and these fronts travel through the suspension as kinematic waves [12]. Hence, the layers are distin-

99 guished by mass fraction  $\chi_j$ , of solid material in the  
100 sediment. The layers' density is described by

$$\rho_j = \chi_j \rho_s + (1 - \chi_j) \rho_w, \quad (1)$$

102 where the index  $j$  numerates the layers,  $\rho_s$  is the  
103 suspended solid material density and  $\rho_w$  is the sus-  
104 pending fluid density (corresponding to either fresh or  
105 salty water). We consider a simple configuration of  
106 two neighboring sediment layers  $j=1,2$  (layer-1  
107 underlies layer-2 so that  $\rho_1 > \rho_2$ , Fig. 1a) with thick-  
108 ness  $H$  (A typical value of  $\rho_s = 2500 \text{ kg/m}^3$  where the  
109 water density might vary between  $\rho_w \sim 1000\text{--}1300 \text{ kg/}$   
110  $\text{m}^3$ , for fresh and salty water. A typical value of the  
111 fraction in the sediments is  $\chi = 2/3$  and thus Eq. (1)  
112 suggests a mean typical density value of the sediments  
113 at the range of  $\rho_m = 2000\text{--}2100 \text{ kg/m}^3$ . The typical  
114 fraction difference between two successive sediment  
115 layers  $\Delta\chi = \chi_1 - \chi_2$ , is of the order of 0.1 which gives  
116  $\Delta\rho = 150\text{--}120 \text{ kg/m}^3 \ll \rho_m$ , for both fresh and salty  
117 water.). Observations indicate that the typical unstable  
118 perturbed wavelength is small compared to the layer  
119 interface length but has the same order of the sediment  
120 layer width (aspect ratio at the order of unity) [2].  
121 Hence we take for simplicity an infinite horizontal  
122 interface (with no vertical boundaries). We consider a  
123 case where away from the interface at say  $z = \pm H$  the  
124 perturbation vertical velocity vanishes. We assume  
125 that the problem is essentially two dimensional  
126 (where  $x$  is the direction of the earthquake shaking  
127 and  $z$  is the vertical), hydrostatic, incompressible and  
128 irrotational away from the interface.

129 Introducing viscosity to the problem is non-trivial  
130 since it is impossible to recover from the present  
131 folded sediment layers the original viscosity qualities  
132 of the paleo unconsolidated mud before deformation.  
133 Moreover, the effective viscosity of a thick suspension  
134 under dynamic conditions depends on sizes and  
135 shapes of suspended particles. These properties are  
136 not well characterized in many natural deposits, and  
137 their quantitative effect on viscosity is poorly known.  
138 Even if we assume an isotropic viscosity within the  
139 layers it is straightforward to show that the viscosity  
140 vanishes for an incompressible irrotational flow. Then  
141 the viscosity should be incorporated in the internal  
142 boundary condition by requiring continuity of the  
143 normal stress along both sides of the interface [1].  
144 These normal stress cannot be quantified however,

145 from present observations. Hence, here we take a  
146 simple approach of representing viscosity in terms  
147 of the bulk Rayleigh damping [13]:

$$\mathbf{f}_v = -r\mathbf{u}, \quad (2)$$

148 where  $\mathbf{f}_v$  provides the damping force per unit mass, **149**  
 $\mathbf{u} = (u, 0, w)$  is the 2-D velocity vector and for a given  
150 dominant frequency  $r$  is taken as a constant. Damping  
151 should be sufficient to reduce the motion significantly  
152 within the time scale of an earthquake duration, how-  
153 ever it should not be too strong as to diminish the  
154 motion completely. We can estimate the damping by  
155 using the response to seismic shear waves, as the  
156 attenuation of these would dissipate energy in a man-  
157 ner similar to that of internal gravity waves [14]. The  
158 quality factor,  $Q$ , is the ratio between the stored  
159 energy and the energy lost during a cycle. Due to  
160 the Rayleigh damping the wave amplitude decays as  
161  $\exp(-rt)$  and its energy as  $\exp(-2rt)$ . Hence,  
162  $Q = 2\pi/[1 - \exp(-2r/f)] \approx \pi f/r$ , were  $f$  represents  
163 the frequency of the most energetic wave, if we  
164 assume  $r/f \ll 1$ . Recent estimates based on in situ  
165 measurements for sediments [15], provide typical  
166 values of  $Q \sim 30 \pm 20$ , thus suggesting  $r \sim 0.1f$ . **167**

168 We treat the acceleration perturbation of earthquake  
169 waves in the soft sediment as pressure gradients, with a  
170 horizontal component  $\Pi = -\frac{\partial p}{\partial x}$ . The pressure gradi-  
171 ent force is assumed to be damped by Eq. (2), within  
172 the time scale of the duration of strong motion. As a  
173 result the layers reach an approximate balance where  
174 both layers move in concert but the denser lower layer  
175 moves more slowly than the upper one, i.e. **175**

$$rU_j = \frac{\Pi}{\rho_j} \quad (3)$$

176 ( $U_j$  denotes the mean velocity of layer  $j$ ), forming **176**  
177 shear at the interface. We are focusing on cases where  
178 hindered settling creates minor differences between  
179 adjacent layers (Appendix A) hence we assume that  
180  $\Delta\rho \ll \rho_m = (\rho_1 + \rho_2)/2$ . Then Eq. (3) gives **181**

$$\Delta U = \frac{\Pi}{r\rho_m^2} \Delta\rho, \quad (4)$$

182 where  $\Delta U = U_2 - U_1 > 0$  and  $\Delta\rho = \rho_1 - \rho_2 > 0$ . **183**

184 Hence, under these simplified assumptions a Kel-  
185 vin–Helmholtz like configuration of stratified sheared  
186 bi-layer is being established within the time scale of **186**

187 an earthquake. Next we examine the possible modal  
188 instability resulted from the KHI mechanism.

### 189 3. Damped growth of sediment KH

190 We seek normal mode wavelike solutions for the  
191 perturbation in the form of  $\exp[ik(x - ct)]$ , where  $k$  is  
192 wavenumber and  $c$  is phase speed (which could be  
193 complex). Then in the Appendix we derive the  
194 damped bounded KHI dispersion relation,

$$c = U_m + i \left( \frac{rH}{2K} \right) \left\{ \pm \left[ 1 + \left( \frac{A}{r} \right)^2 K(K - 2Ri \tanh K) \right]^{1/2} - 1 \right\} \quad (5)$$

196 where

$$U_m = \frac{\rho_1 U_1 + \rho_2 U_2}{\rho_1 + \rho_2}, \quad K = kH, \\ A = \frac{\Delta U}{H}, \quad N^2 = \frac{g}{\rho_m} \frac{\Delta \rho}{H}, \quad Ri = \left( \frac{N}{A} \right)^2. \quad (6a, b, c, d, e)$$

198  $U_m$  is density weighted mean velocity,  $K$  is the  
200 nondimensional wavenumber scaled by the layers'  
201 width  $H$ ,  $A$  can be regarded as the bulk mean shear  
202 and  $N$  as the bulk buoyancy (Brunt–Väisälä) frequen-  
203 cy. The square of the ratio of the two latter terms is  
204 known to be the bulk Richardson number [13].

205 Modal instability is obtained when the imaginary  
206 part of the phase speed,  $c_i$ , of Eq. (5) is positive,  
207 possible only for wavenumbers  $K > K_c = 2Ri \tanh$   
208  $K_c$ , which is precisely the explicit criterion for the  
209 inviscid case of bounded KHI. Using Eq. (6c,e), the  
210 latter condition can also be rewritten in terms of the  
211 minimal shear required to make a specific wavelength  
212 unstable in a given density stratification, i.e.,  
213  $\Delta U > NH \sqrt{2 \tanh K/K}$ . The difference between the  
214 viscid and the inviscid KHI instability is therefore not  
215 in the range of instability but in the exponential  
216 growth rate,  $GR = kc_i s^{-1}$ ,

$$GR = \frac{r}{2} \left\{ \left[ 1 + \left( \frac{A}{r} \right)^2 K(K - 2Ri \tanh K) \right]^{1/2} - 1 \right\} \quad (7)$$

218 which is always smaller than the inviscid KHI growth  
219 rate (when  $r=0$ ).

220 A useful measure for earthquake effectiveness is  
221 the ground acceleration imposed by the shaking, com-  
222 monly normalized by the gravitation acceleration  $g$ .  
223 Hence, defining the averaged ground acceleration as  
224  $a = \Pi / \rho_m$ , then using Eqs. (4) and (6c), the condition  
225 for instability can be rewritten as  $a/g > (r/N)$   
226  $\sqrt{2 \tanh K/K}$ . The typical perturbed wavelength  
227 which is found in the observations (c.f. Fig. 1) has  
228 an aspect ratio around unity, i.e.,  $\lambda/H \sim 1$  or  $K \sim 2\pi$   
229 and  $\tanh(K) \approx 1$ . Therefore, the lower limit to the  
230 averaged ground acceleration for the development of  
231 such perturbations is

$$\frac{a}{g} > \frac{r}{\sqrt{\pi N}} = r \sqrt{\frac{\rho_m H}{\pi g \Delta \rho}}. \quad (8)$$

232 The threshold for instability increases with  
233 damping  $r$  and with the square-root of the layer  
234 thickness. The threshold is inversely proportional to  
235 the square-root of the density difference suggesting  
236 that a high density difference is less stable. By  
237 contrast, density difference tends to suppress the  
238 inviscid KHI instability. This somewhat surprising  
239 result for the viscid case considered here is solely  
240 due to the increasing bulk shear for a given pres-  
241 sure gradient. The growth rate Eq. (7) then takes the  
242 form  
243  
244

$$GR = \frac{r}{2} \left\{ \left[ 1 + \left( \frac{A}{r} \right)^2 4\pi(\pi - Ri) \right]^{1/2} - 1 \right\}. \quad (9)$$

245 Finally, using Eqs. (4) and (6), then Eq. (9) can be  
246 rewritten explicitly in terms of  $(H, a/g)$

$$GR = \frac{r}{2} \left\{ \left[ 1 + \left( \frac{2\pi g \Delta \rho}{r^2 \rho_m H} \right)^2 \left( \frac{a}{g} \right)^2 - \frac{r^2 \rho_m}{\pi g \Delta \rho} H \right]^{1/2} - 1 \right\}. \quad (10)$$

250 Fig. 2 shows the calculated growth rate (normal-  
251 ized by the frequency of the dominant seismic wave)  
252 as a function of  $(H, a/g)$ . For this example we take  
253 typical values of the Dead Sea sediment composi-  
254 tion. The solid material, with density  $\rho_s = 2500 \text{ kg/}$   
255  $\text{m}^3$ , is suspended into salty water with density  
256  $\rho_w = 1200 \text{ kg/m}^3$ . The averaged suspended mass frac-  
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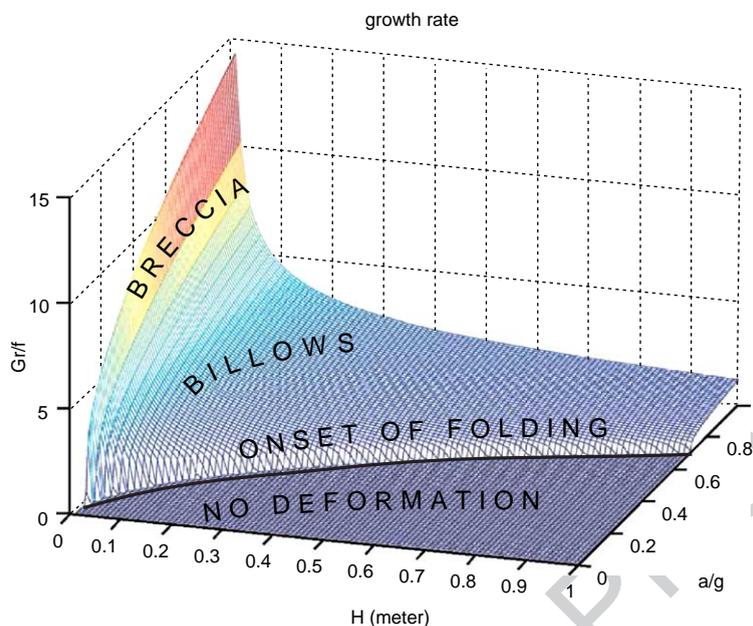


Fig. 2. An example for the KHI growth rate, normalized by frequency  $f$ , as a function of the layer thickness  $H$ , and the normalized ground acceleration  $a/g$ , as given by (10). Here we take typical values of the Dead Sea sediment composition (c.f. text) of  $\rho_m = 2000 \text{ kg/m}^3$ , and  $\Delta\rho = 130 \text{ kg/m}^3$ . The damping coefficient is taken as  $r = 0.1f$ . The parabolic solid line marks the threshold for instability (c.f. 8). For onset of linear KHI wave folding (as in Fig. 1a), the growth rates must be in the order of the gravest seismic wave frequency (order of 1 Hz). Coherent billows (Fig. 1b) require high growth rates, where fully turbulent mixing, leading to breccia layers (Fig. 1c), requires yet higher growth rates.

258 tion,  $\chi_m$ , and its difference between the layers,  $\Delta\chi$ ,  
 259 are  $2/3$  and  $0.1$ , respectively, yielding  $\rho_m \approx 2000$   
 260  $\text{kg/m}^3$ , and  $\Delta\rho = 130 \text{ kg/m}^3$ . The damping coefficient  
 261 was taken as  $r = 0.1f$ , where the frequency of the  
 262 most energetic seismic waves  $f$  was taken as  $1 \text{ Hz}$ .  
 263 The layers are stable for ground accelerations  $a$ ,  
 264 smaller than  $0.07\sqrt{Hg}$  (where  $H$  is in meters), c.f.  
 265 the parabolic threshold solid line Eq. (8). For onset  
 266 of linear KHI wave folding (as in Fig. 1a), the growth  
 267 rates must be in the order of  $f$ . Coherent billows  
 268 (Fig. 1b) require high growth rates, where fully  
 269 turbulent mixing, leading to breccia layers (Fig.  
 270 1c), requires yet higher growth rates. For example,  
 271 for an acceleration  $a = 0.7g$ , layers of  $1 \text{ m}$  thickness  
 272 or thicker are stable. Thinner layers become unstable,  
 273 yet for thickness larger than about half a meter, the  
 274 growth rate might not suffice for the development of  
 275 instability during the seismic wave half-period. Bil-  
 276 low structures can develop in layers with a thickness  
 277 of fractions of a meter, while fully turbulent mixing  
 278 is expected for layers with thickness of the order of  
 279 centimeters.

#### 4. Discussion

Laminated fine-grained sediments, deposited on  
 horizontal bottom under low-energy conditions, are  
 expected to be stably stratified, so density increases  
 with compaction and hence with depth. Since gravi-  
 tational Rayleigh–Taylor instabilities are not likely  
 under these conditions, alternative mechanisms  
 should be at work. From an observational aspect,  
 the striking similarity of structures in fine-grained  
 laminated deposits to KHI billows, suggests that  
 shear plays a central role in soft sediment deformation  
 (For example, Fig. 3 is a photograph of KHI billow  
 clouds taken in New Zealand during the summer. An  
 atmospheric inversion layer yields both strong strati-  
 fication and wind shear which together enable KHI  
 instability to develop. Please note the similarity be-  
 tween Figs. 1b and 3.).

The simple analysis presented here examines the  
 feasibility of KHI instability in stably stratified sedi-  
 ments. We contend that shear energy is available in  
 various depositional settings such as sloping substrate,

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Fig. 3. Kelvin Helmholtz billow clouds in the New Zealand summer sky. The clouds are formed along an atmospheric inversion layer (of strong stratification, where temperature increases with height). The inversion layer tends to isolate the surface boundary layer wind from the free atmosphere above it and as a result a shear is formed along the inversion layer. The combination of shear, stratification and humidity provides the conditions for KHI billow clouds to develop. The photo was taken by Mr. Attay Harkabi on January 2001.

301 high water energy environment, or earthquake prone  
 302 regions. We focus here on earthquake related settings,  
 303 yet the analysis is valid for other scenarios, as long as  
 304 the pressure gradients are known.

305 The application of the model to earthquake settings  
 306 is based on a translation of the instrumentally measur-  
 307 able ground accelerations to pressure gradients. As a  
 308 preliminary model it does not provide precise corre-  
 309 spondence between field observations and the actual  
 310 driving ground accelerations. Moreover, we cannot  
 311 rule out alternative sources for pressure gradients,  
 312 such as surface and internal waves in the depositing  
 313 water body. Since water depth of Lake Lisan was  
 314 several tens of meters, ground acceleration waves dom-  
 315 inated over water waves.

316 In deriving the model we have neglected differential  
 317 displacement between grains and suspending fluid.  
 318 Such displacement is evident in the very existence of  
 319 billows in sedimentary rocks. While KHI instabilities  
 320 are very common in our surroundings, the resulting  
 321 structures are ephemeral due to the stabilizing force of  
 322 gravity. By contrast, billows are so well preserved in  
 323 sediments due to water loss and consolidation shortly  
 324 after the onset of the instability. We assume here that on  
 325 the time scale of development of KHI instability, the  
 326 differential displacement between grains and suspending  
 327 fluid is negligible, yet in the time scale of attenu-  
 328 ation of the seismic energy, the differential  
 329 displacement is sufficient to preserve some of the  
 330 structure.

331 Tentative evidence for such differential displace-  
 332 ment can be seen in Fig. 1c. [5] have interpreted such

structures as a case for differential displacement be- 333  
 tween grain and water during shaking. Water loss in 334  
 the lower cohesive zone to the overlying homoge- 335  
 neous zone may have provided upward flow to 336  
 drive complete suspension of this zone. 337

The analysis assumes that all displacement fields 338  
 are confined to a vertical plane. This assumption is 339  
 valid away from the earthquake source, where  $P$ ,  $S$ , 340  
 and surface waves have dispersed sufficiently. Some 341  
 of the most photogenic cases of billows in the Dead 342  
 Sea laminated deposits indeed show such two-dimen- 343  
 sional displacement fields. In the vicinity of the earth- 344  
 quake source, the displacement field is three 345  
 dimensional, and a more complete analysis is re- 346  
 quired. At the same time, well documented three 347  
 dimensional observations will be useful for the 348  
 study of the vicinity of the earthquake source. 349

The introduction of bulk damping to represent 350  
 viscosity, a key simplification of the present analysis, 351  
 assumes that the viscous damping of the suspension 352  
 is proportional to the velocity. This representation 353  
 bypasses the formidable challenge of assessing the 354  
 effective viscosity under the dynamic conditions of 355  
 shaking of a thick suspension, in which the sus- 356  
 pended particles exhibit a wide range of shapes 357  
 and scales [16]. The advantage in the present formu- 358  
 lation is that the actual coefficient of damping can be 359  
 estimated from direct experimental observations in a 360  
 shaking tank. In the absence of such experimental 361  
 data, we parametrize the bulk Rayleigh damping 362  
 coefficient in terms of the driving frequency domi- 363  
 nant in the acceleration spectrum of the earthquake. 364

365 The time scale for damping should be sufficiently  
 366 short as to balance the driving pressure gradient  
 367 during the time around peak acceleration, say a  
 368 tenth of the acceleration cycle. This is the rationale  
 369 behind choosing a bulk Rayleigh friction coefficient  
 370 10 times the frequency. The estimation of damping is  
 371 also in agreement with the attenuation of seismic  
 372 shear waves, as the attenuation of these would dis-  
 373 sipate energy in a manner similar to that of internal  
 374 gravity waves [14].

375 The force balance between the driving pressure  
 376 gradient and Rayleigh damping yields an expres-  
 377 sion for the mean shear between the two layers Eq.  
 378 (4), turning out proportional to the density differ-  
 379 ence. This leads to the somewhat nonintuitive result  
 380 that damped KHI instability increases with the den-  
 381 sity difference Eq. (8). This result is only valid in  
 382 the range of small density difference (with respect  
 383 to the mean density), a range in which Eq. (4)  
 384 results from Eq. (3). Hence high density difference  
 385 cannot lead to instability at low acceleration. The  
 386 prediction of Eq. (8), namely that the instability is  
 387 promoted by increasing the density difference (at the  
 388 low range of  $\Delta\rho/\rho_m$ ) can be subjected to experi-  
 389 mental verification.

390 The ability of the analysis to rationalize field  
 391 observations suggests that KHI instability is a plau-  
 392 sible mechanism for deformation of stably stratified  
 393 soft sediments. Earthquake-triggered KHI instability  
 394 seems plausible for deposits laid horizontally in  
 395 calm water, as the case is for the earthquake  
 396 prone Dead Sea basin. The present study sets a  
 397 foundation for quantitative analysis of deformation  
 398 structures in laminated sediments and for the extrac-  
 399 tion of dynamic conditions during earthquakes. This  
 400 can be a contribution to earthquake science and  
 401 hazard assessment.

402 Our analysis indicates that density inversion is  
 403 not required from the physics of earthquake-induced  
 404 soft sediment deformation. By extension, we expect  
 405 that the Kelvin–Helmholtz instability will provide  
 406 explanations to common geophysical situations,  
 407 where gravitational instabilities are inhibited by  
 408 density stratification. These may include the em-  
 409 placement of ophiolites, mixing of the upper mantle  
 410 with the denser lower mantle, and entrainment by  
 411 hot plumes of dense slugs at the core mantle  
 412 boundary.

## Appendix A. Bounded modal KHI instability in the presence of Rayleigh damping: 413 414

Writing the 2-D Navier–Stokes Eq. (2). 415

$$\frac{\partial \mathbf{u}}{\partial t} + \omega \times \mathbf{u} + \nabla \frac{|\mathbf{u}|^2}{2} = -\frac{1}{\rho} \nabla p - \nabla \Phi + \mathbf{f}, \quad (\text{A.1})$$

the vorticity  $\omega = \nabla \times \mathbf{u}$ ,  $p$ ,  $\rho$ ,  $\Phi = gz$ , are the pressure, 416  
 density and the gravitation potential.  $t$  denotes time 418  
 and  $\nabla = (\partial_x, 0, \partial_z)$ . The viscosity is given by Eq. (2). 419  
 We assume a mean hydrostatic balance in the vertical 420  
 direction and a mean horizontal balance in the form of 421  
 Eq. (3). 422

We seek normal mode wavelike solutions for the 423  
 perturbation in the form of  $\exp[ik(x - ct)]$ . Every- 424  
 where except at the interface, the layers are assumed 425  
 irrotational,  $\omega = 0$ . While the normal modes result 426  
 from the basic state vorticity  $\delta$ -function on the inter- 427  
 face, the companion set of solutions of the continuous 428  
 spectrum results from rotation within the layers. The 429  
 continuous spectrum is neutral and therefore tradi- 430  
 tionally neglected in the context of linear stability. 431  
 However, due to non-orthogonality between the con- 432  
 tinuous spectrum and the normal modes, an interac- 433  
 tion between the two sets of solutions might lead to a 434  
 super non-modal growth in finite time [17]. This sort 435  
 of growth mechanism is however beyond the scope of 436  
 this work. The modal velocity can be written then in 437  
 terms of the velocity potential  $\psi$ ;  $\mathbf{u} = -\Delta\psi$ . Then Eq. 438  
 (A.1) can be rewritten in the barotropic gradient form: 439

$$\nabla \left[ -\left( \frac{\partial \psi}{\partial t} + r\psi \right) + \left( \frac{|\mathbf{u}|^2}{2} + \frac{p}{\rho} + \Phi \right) \right] = 0, \quad (\text{A.2})$$

and Eq. (2) has been used to represent the viscosity. 440  
 Eq. (A.2) implies that 442

$$-\left( \frac{\partial \psi}{\partial t} + r\psi \right) + \left( \frac{|\mathbf{u}|^2}{2} + \frac{p}{\rho} + \Phi \right) = F(t), \quad (\text{A.3})$$

where  $F(t)$  is some function of time only. The LHS of 443  
 Eq. (A.3) can be regarded as the Rayleigh viscid 445  
 unsteady flow generalization of the Bernoulli conser- 446  
 vation (indicated by the second brackets of the LHS). 447  
 Assuming also incompressibility of the layers 448  
 ( $\nabla \cdot \mathbf{u}_i = 0$ ) and decomposing the perturbation from the 449

450 basic state so that  $\mathbf{u}_i = (U_i + u'_i, 0, w'_i)$ , prime indicates  
 451 the perturbation, then  $\psi_i = -U_i x + \hat{\psi}'_i$ . Writing  
 452  $\psi'_i = \hat{\psi}'_i(z) \exp [ik(x - ct)]$ , the incompressibility con-  
 453 strain yields the Laplace equation for the perturbation  
 454 streamfunction ( $\nabla^2 \hat{\psi}'_i = 0$ ).

455 The boundary conditions of vanishing the vertical  
 456 velocity on the horizontal outer boundaries of the  
 457 layers, located at  $z = \pm H$ , give the solution:

$$\tilde{\psi}'_i(z) = \frac{\hat{\psi}'_i}{\cosh(kH)} \cosh[k(H - |z|)], \quad (\text{A.4})$$

458 where  $\hat{\psi}'_i$  is the perturbation's velocity potential am-  
 460 plitude on the two sides of the interface at  $z=0$ .  
 461 Before and after the time when the perturbation is  
 462 initiated the pressure on both sides of the interface  
 463 should be even. This allows us to set the time function  
 464  $F(t)$  by using the latter condition in Eq. (A.3) at  $z=0$ ,  
 465 prior to the perturbation:

$$\frac{1}{2}(\rho_1 U_1^2 - \rho_2 U_2^2) = (\rho_1 - \rho_2)F(t), \quad (\text{A.5})$$

466 which suggests  $F(t)$  to be taken as constant. Perturb-  
 468 ing the interface with a vertical displacement  $\zeta' = \hat{\zeta}$   
 469  $\exp [ik(x - ct)]$ , linearizing the kinetic energy  $|\mathbf{u}_i|^2/2$   
 470  $\approx U_i^2/2 - \frac{\partial \hat{\psi}'_i}{\partial x} U_i$ , then pressure continuity across the  
 471 perturbed interface, together with Eqs. (A.3-5), yield

$$\begin{aligned} \rho_1 [ik(U_1 - c)\hat{\psi}'_1 + r\hat{\psi}'_1 - g\hat{\zeta}] \\ = \rho_2 [ik(U_2 - c)\hat{\psi}'_2 + r\hat{\psi}'_2 - g\hat{\zeta}]. \end{aligned} \quad (\text{A.6})$$

472 In order to obtain the dispersion relation we also  
 475 imply that the vertical velocity at  $z=0$  is equal to the  
 476 Lagrangian time derivative of the interface vertical  
 477 displacement,

$$w(z=0) = \left( \frac{\partial}{\partial t} + \mathbf{u} \cdot \nabla \right) \hat{\zeta}. \quad (\text{A.7})$$

480 Next we linearize the RHS of Eq. (A.7) with  
 481 respect to the basic state. Recall that  $w = -\partial \psi' / \partial z$ ,  
 482 we can then write for the two sides of the interface

$$\begin{aligned} \tanh(kH)\hat{\psi}'_1 &= -i(U_1 - c)\hat{\zeta}, \\ \tanh(kH)\hat{\psi}'_2 &= -i(U_2 - c)\hat{\zeta}. \end{aligned}$$

483 Using that  $\Delta \rho \ll \rho_m$ , Eqs. (A.6) and (A.8) provide  
 486 the dispersion relation of Eq. (5).

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