

LABORATORY AND FIELD DATA OBSERVATIONS, THEIR IMPLICATION ON THE ESTIMATION OF NONLINEAR SITE RESPONSE

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with the help of Celine Gelis, Julie Regnier, Aurore Laurendeau, Peng-Cheng Liu,
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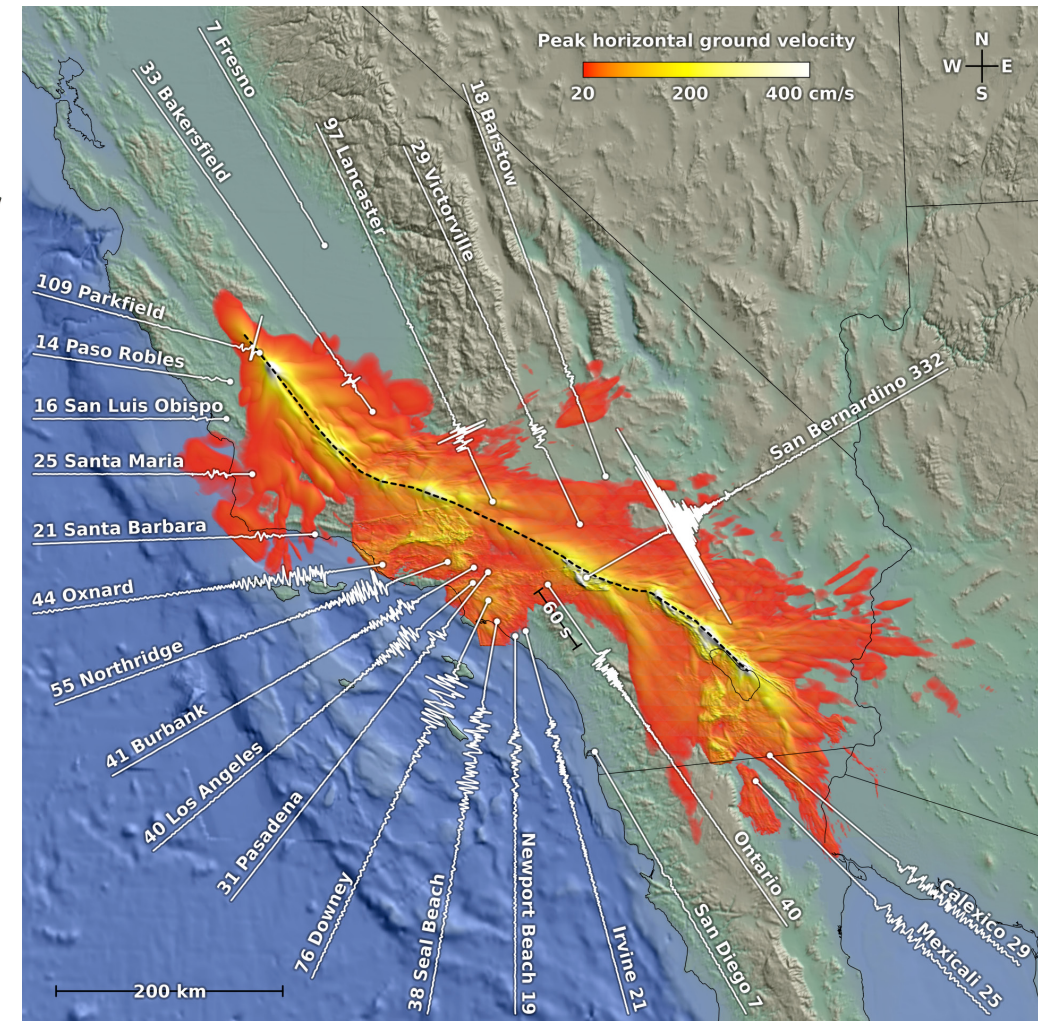
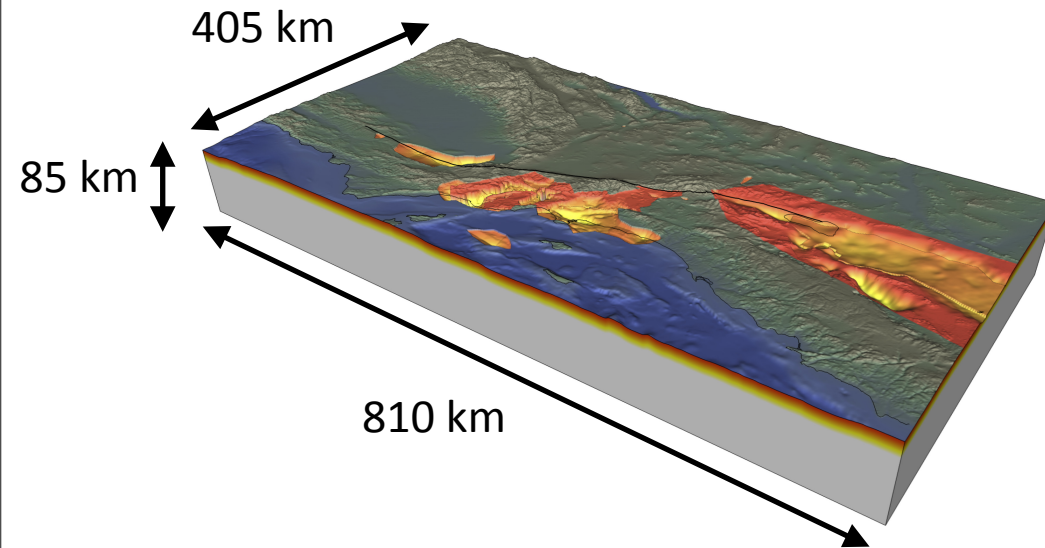


Outline

- Motivation
- Site-specific cases: the 1987 Superstition Hills earthquake
- Widespread nonlinear site response: the 1994 Northridge and the 2011 Tohoku earthquakes
- Lessons learnt from these earthquakes
- Conclusions

Motivation - Seismology

after Cui et al. (2010)

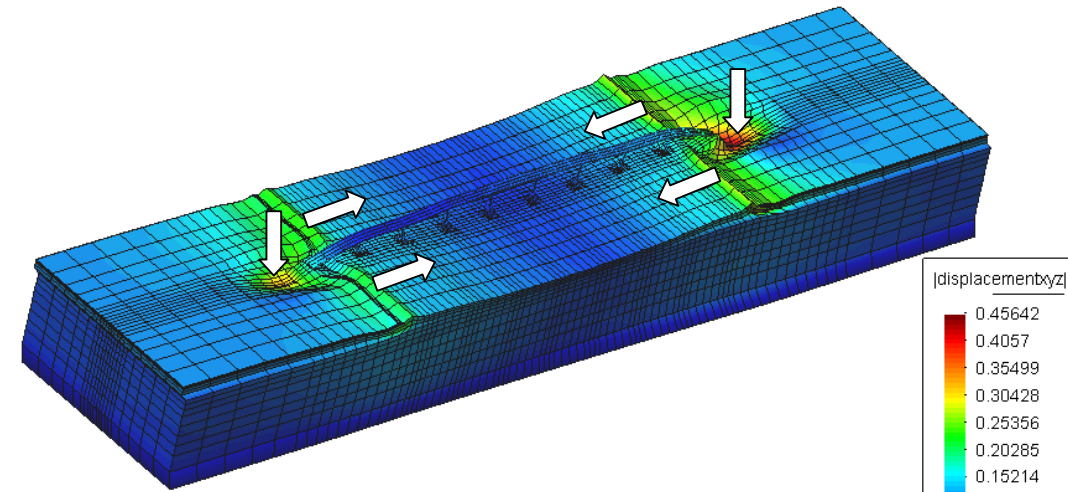
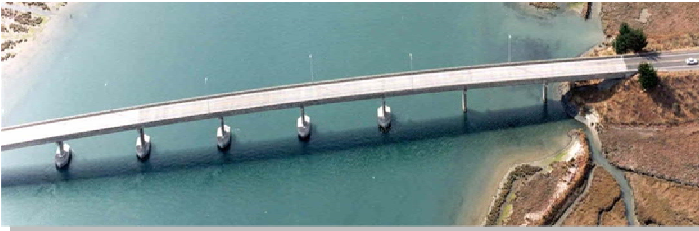


- M8
- 360 s of ground motion
- 436 billion cubic elements
- spontaneous rupture
- minimum $V_s = 400$ m/s
- frequency: 0 - 2 Hz

- Basin effects (PGV = 1 - 4 m/s)
- Directivity and supershear effects
- How might this picture change if soil nonlinearity is taken into account?

Motivation - Earthquake Engineering

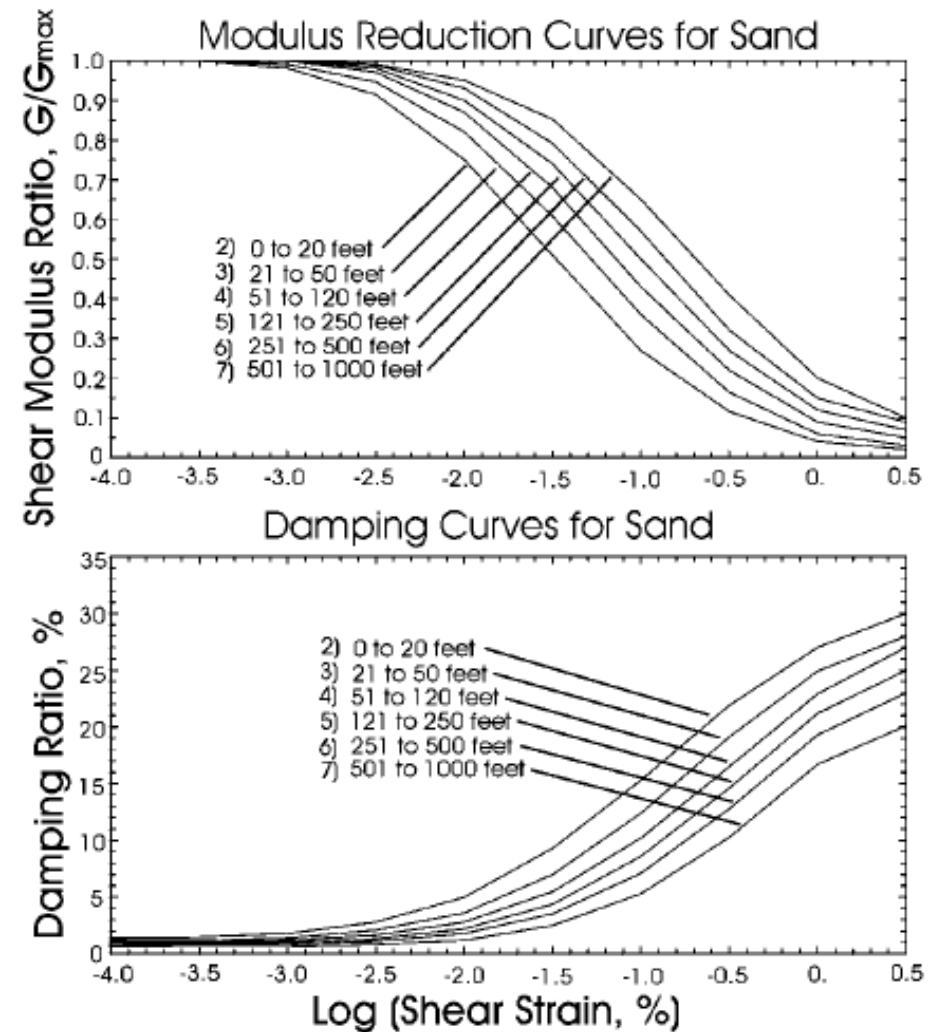
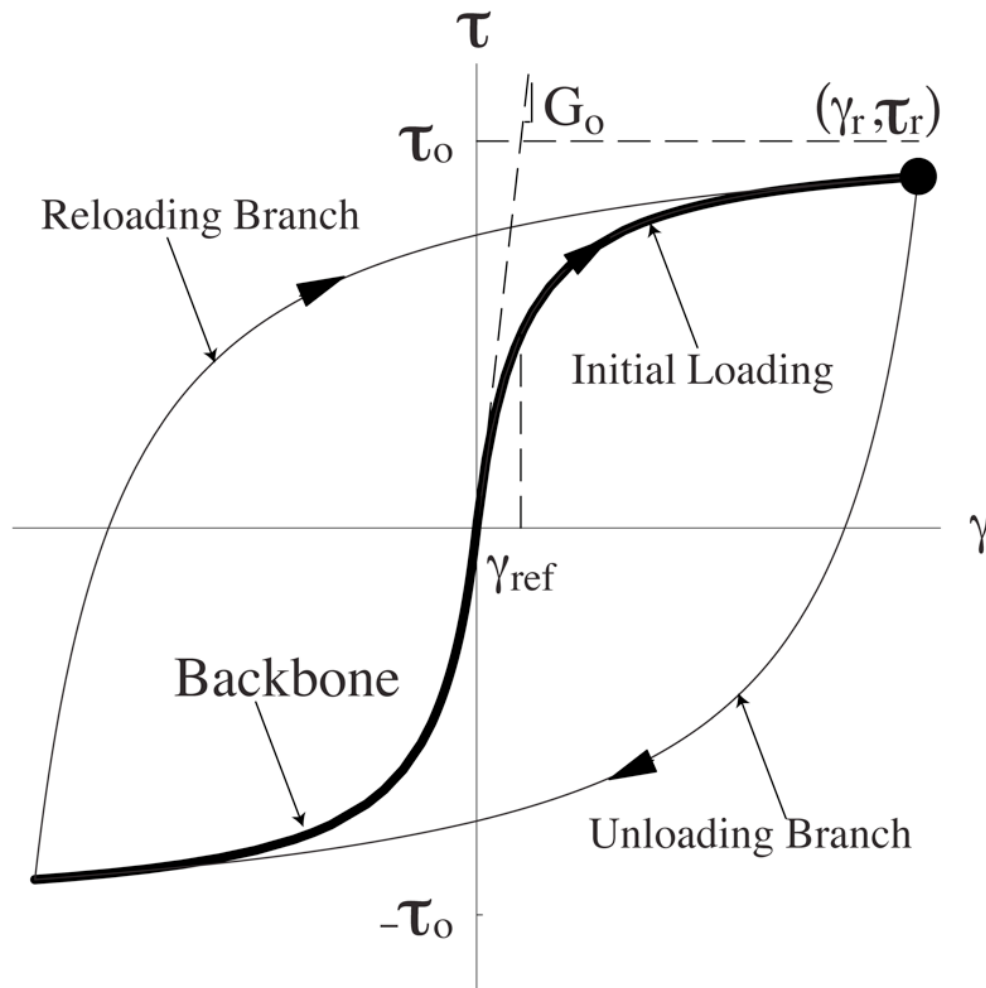
after Elgamal et al. (2008)



- Humboldt Bay Middle Channel Bridge
- 650 x 151 x 74.5 m
- Input: 1978 Tabas earthquake
- soil-structure interaction

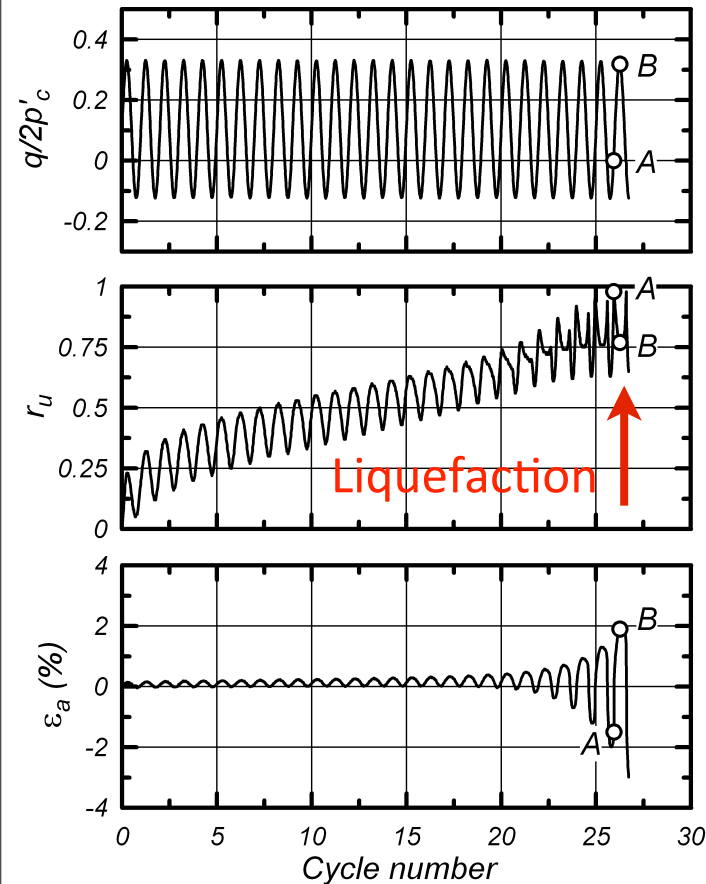
- Distribution of residual settlements of the abutment fill
- Lateral spreading along the river bank
- Bridge deformation
- How might this picture change if local or regional sources were used?

What is nonlinear soil behavior?



- Soils show hysteretic behavior
- Permanent deformations
- Change of resonance frequencies (decreasing shear modulus)
- Deamplification of ground motion (increasing damping)

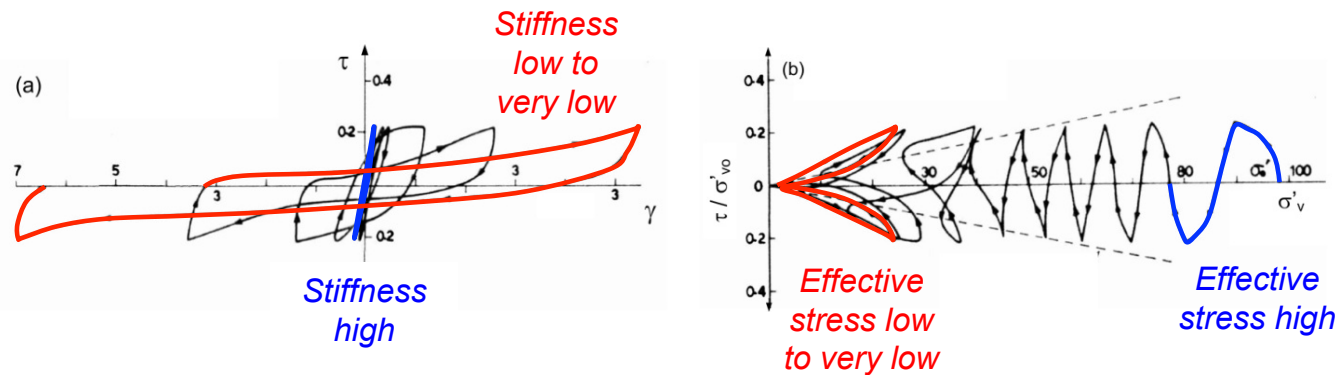
Pore pressure effects (lab data)



Idriss and Boulanger (2006)

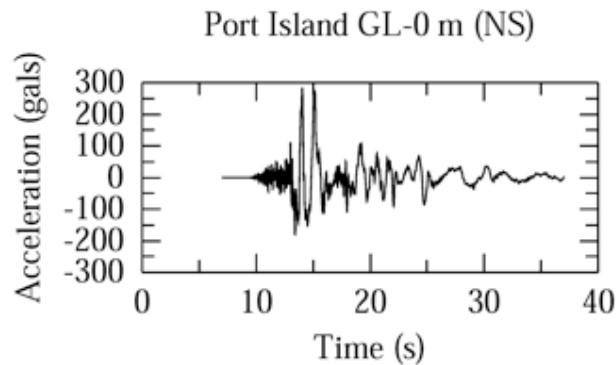
- Stiffness decreases
- Longer period motion
- Lower acceleration amplitudes (increasing damping)
- Higher displacement amplitudes

Ishihara (1985) – Cyclic simple shear test



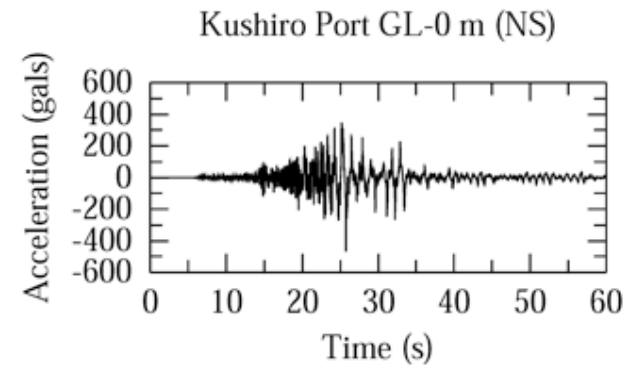
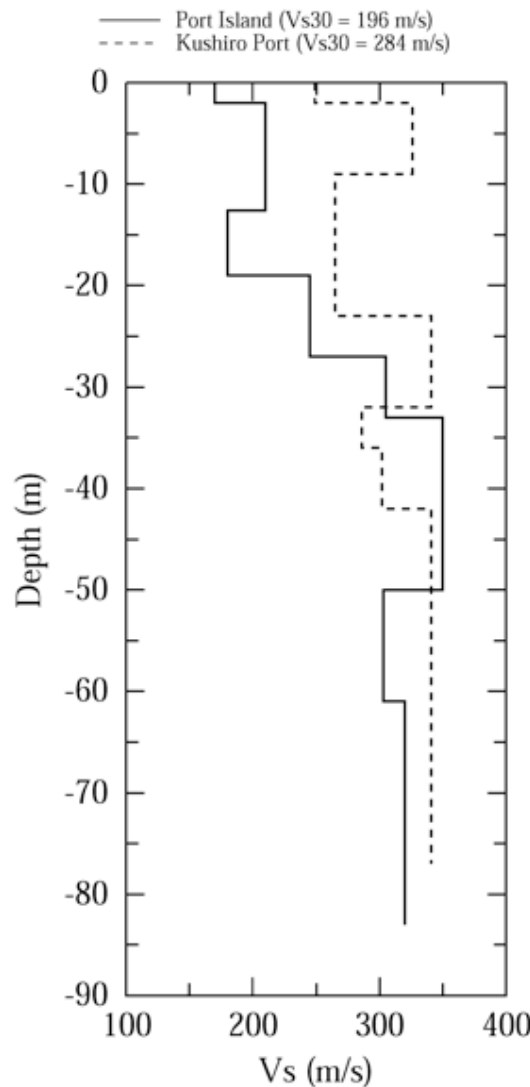
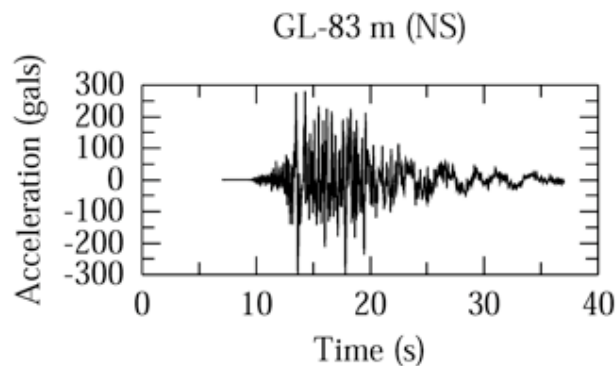
Kramer (2011)

What is the effect on the ground motion?



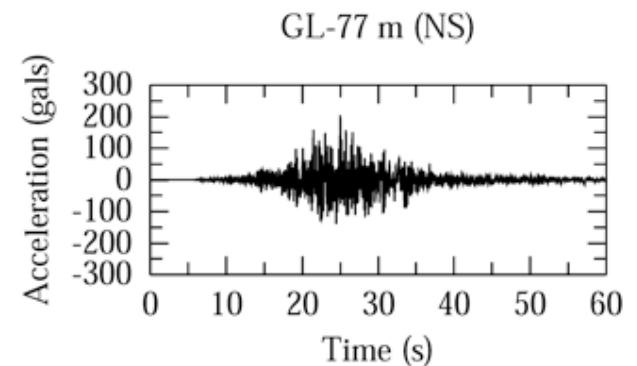
Loose sand
(liquefaction)

- lowpass filtering
- deamplification



Dense sand
(cyclic mobility)

- high frequency content
- amplification

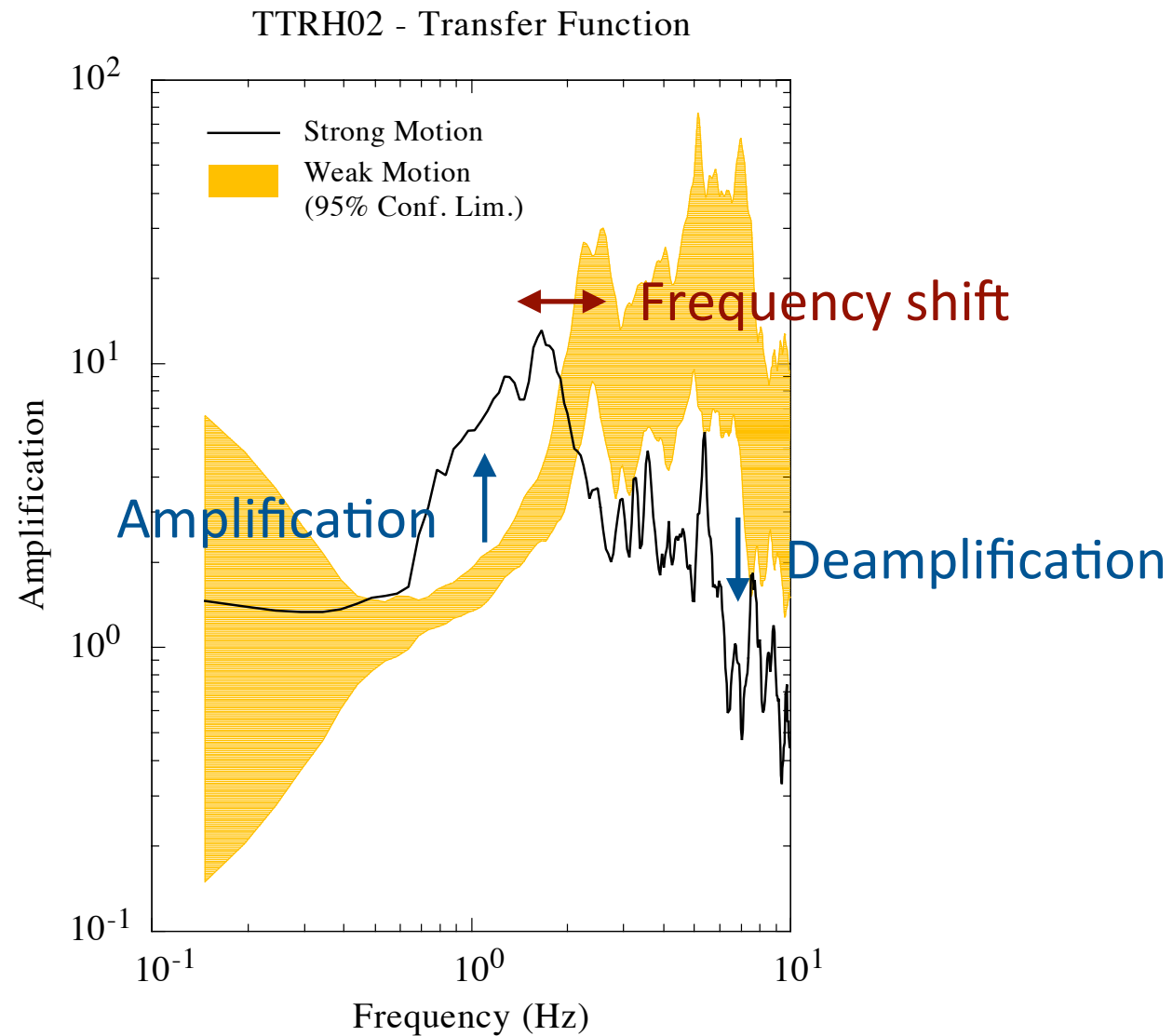


Velocity profile is not enough (elastic parameters)

Seismology: elastic parameters

Earthquake Engineering: dynamic parameters

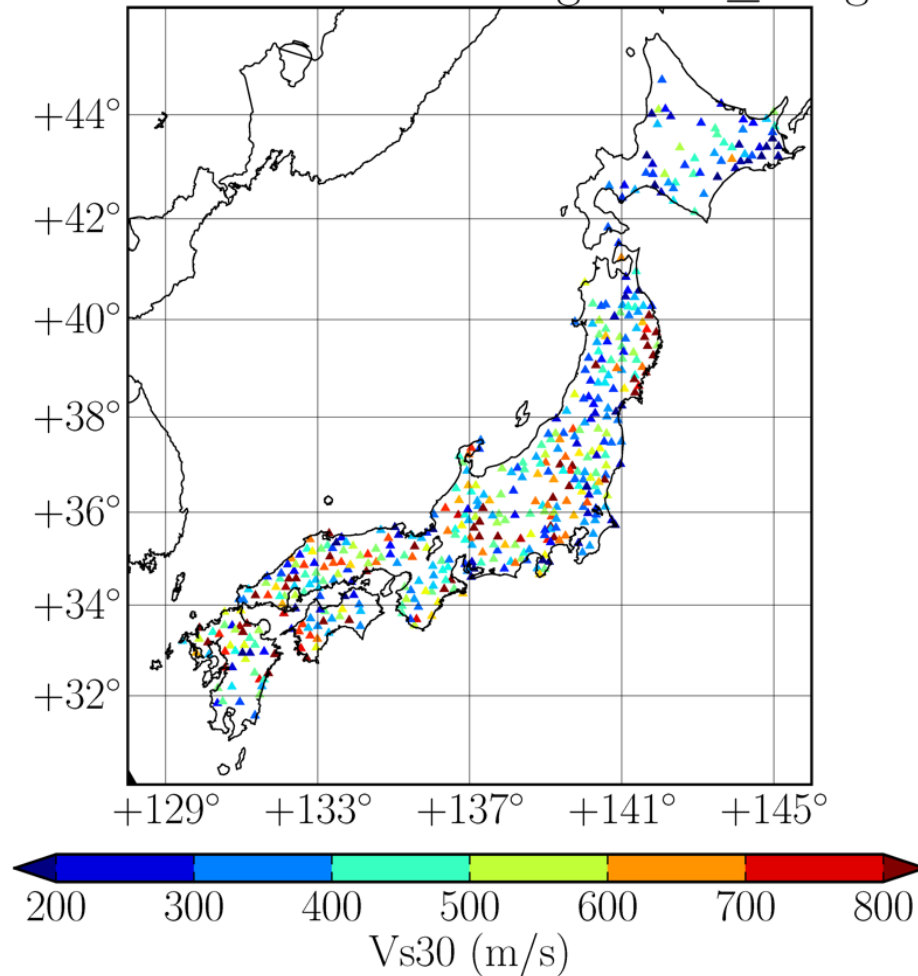
How is soil nonlinearity measured?



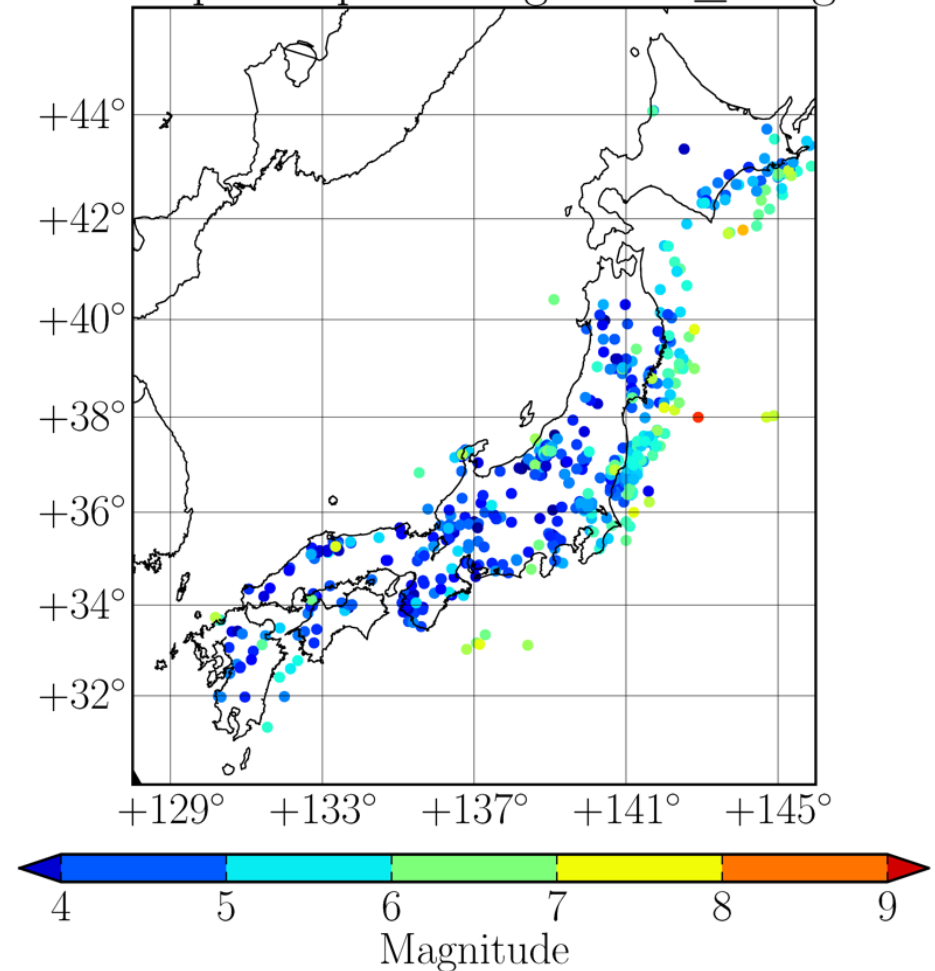
- Linear borehole response using data having PGA < 10 gals
- Nonlinear response using the 2000 Tottori data (M7.3)
- Broadband deamplification and shift to low frequencies

Why do we study Japan?

KiK-net stations recording $\text{PGA} \geq 50$ gals



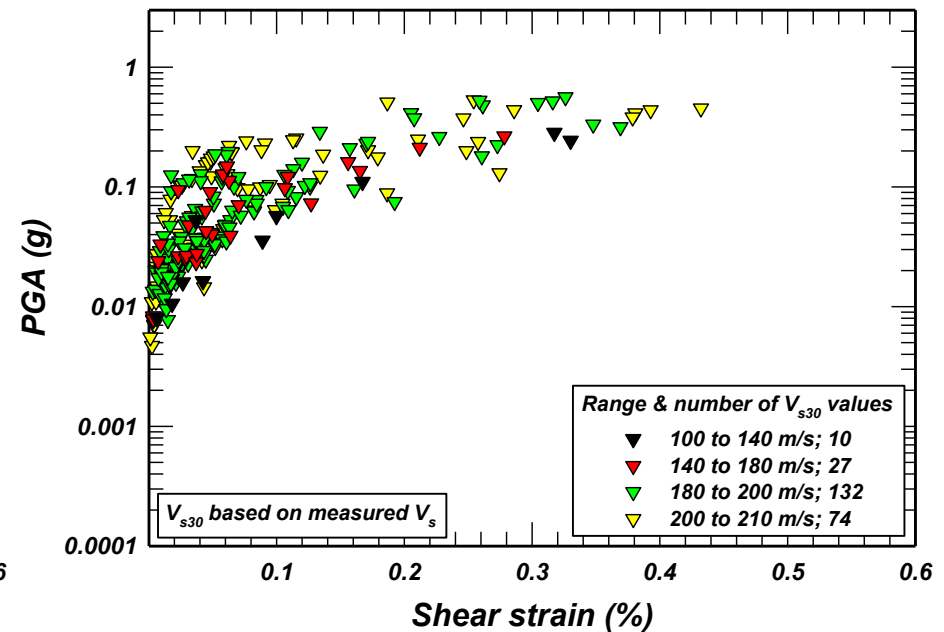
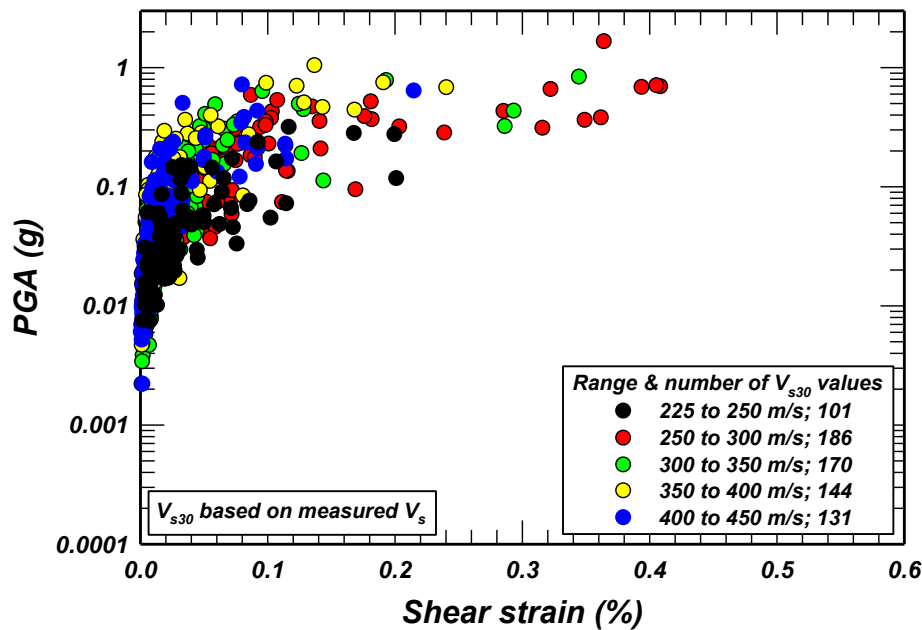
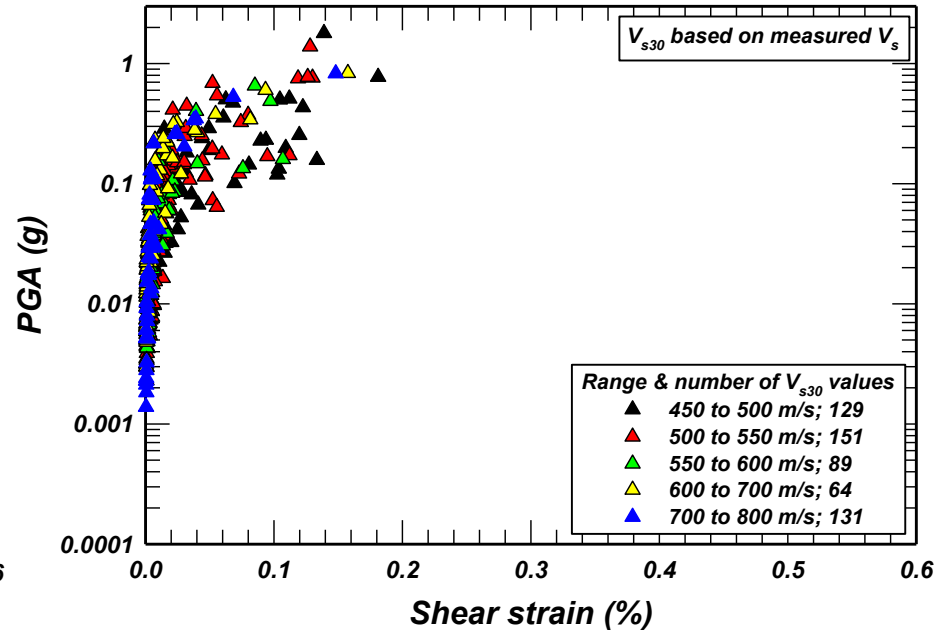
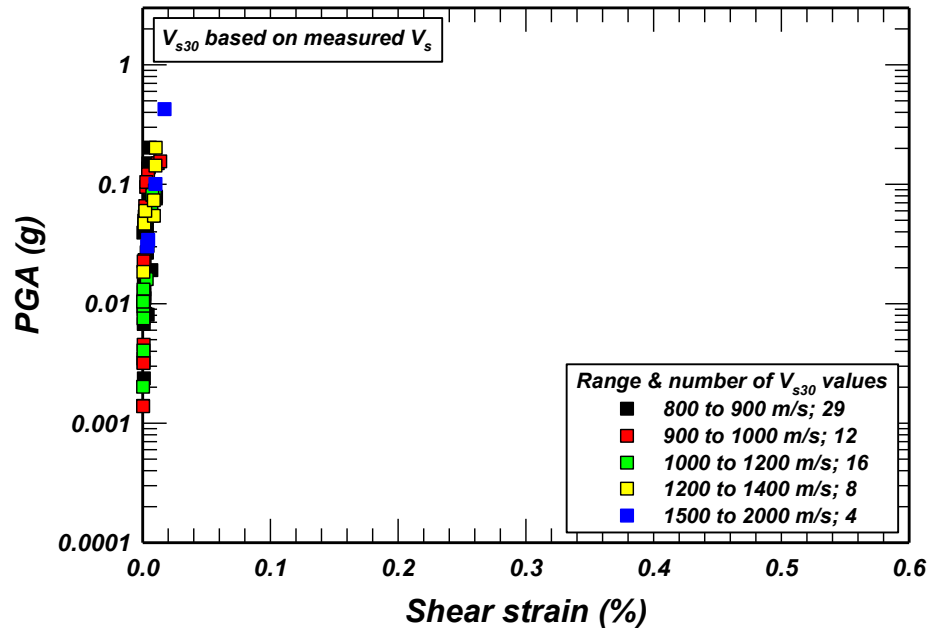
Earthquakes producing $\text{PGA} \geq 50$ gals



- Good station coverage (borehole stations between 100 and 200 m)
- Good earthquake distribution (magnitude and epicentral location)

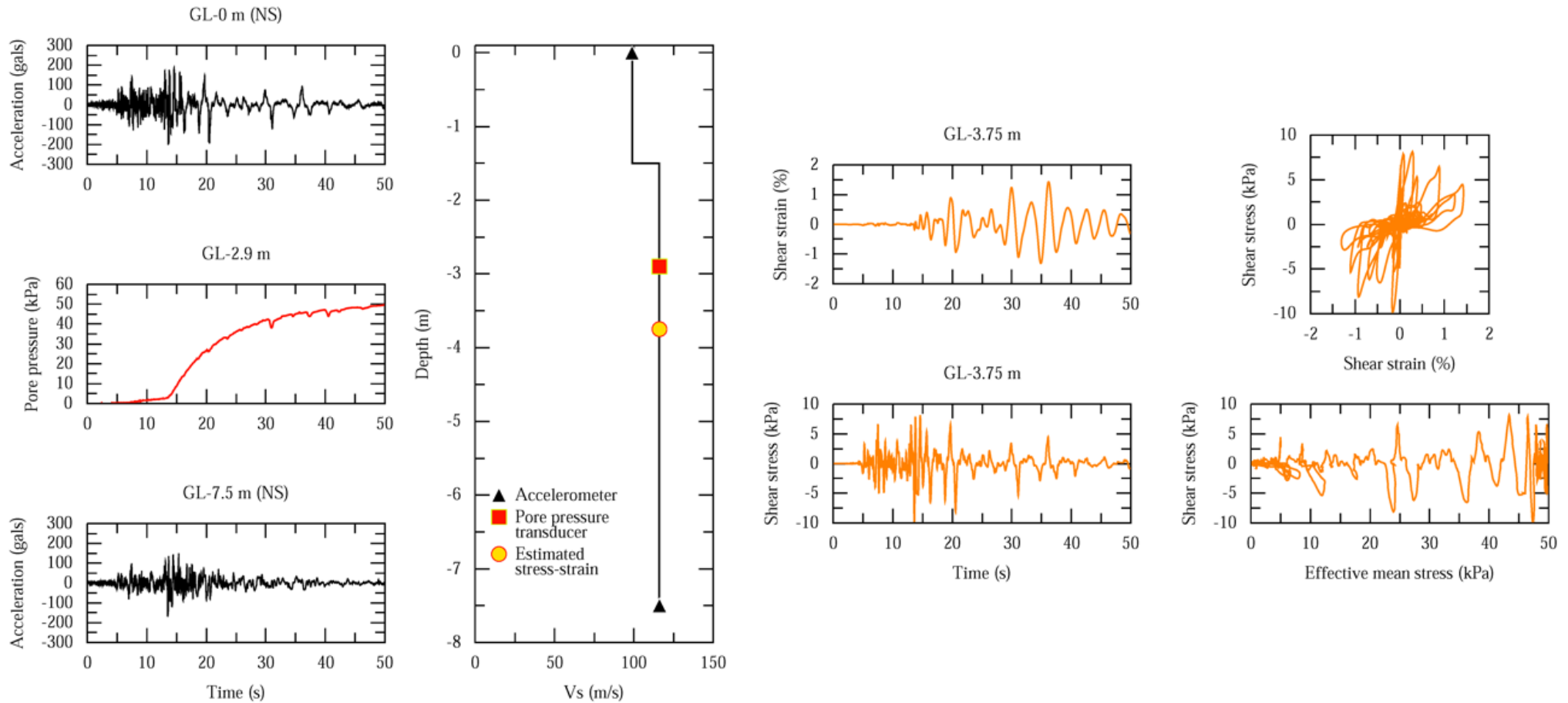
High likelihood of triggering nonlinear soil behavior

Other datasets: the NGA data^{Idriss (2011)}



- PGV/ V_{s30} as a proxy of shear strain
- PGA as a proxy of shear stress

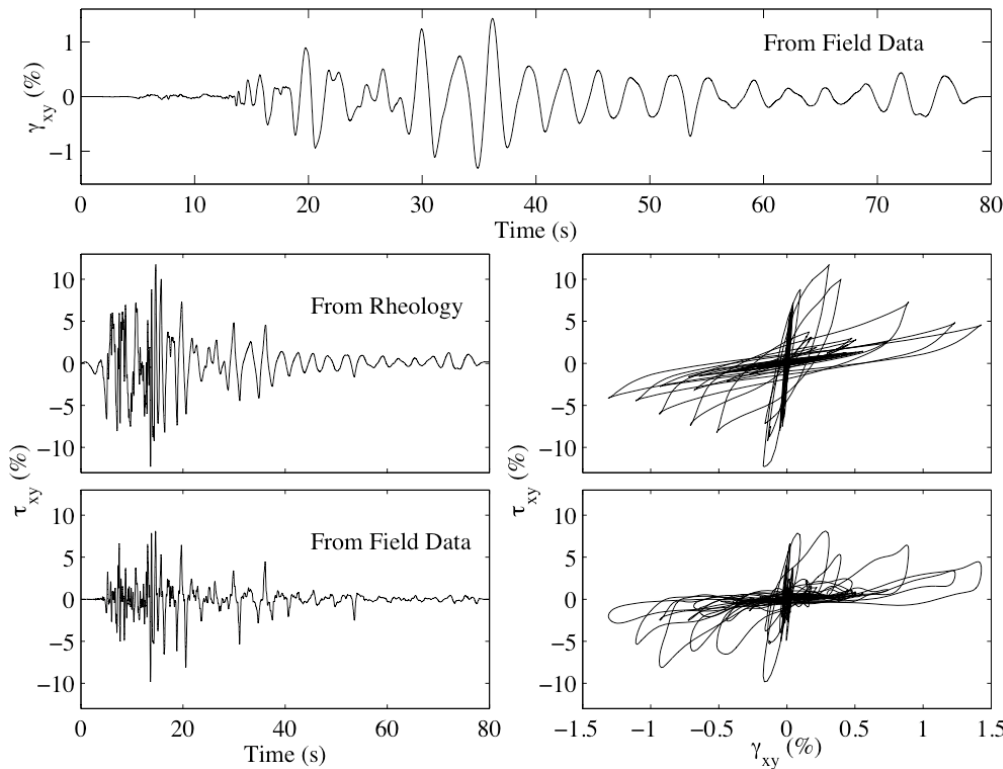
Time domain analyses: site-specific studies



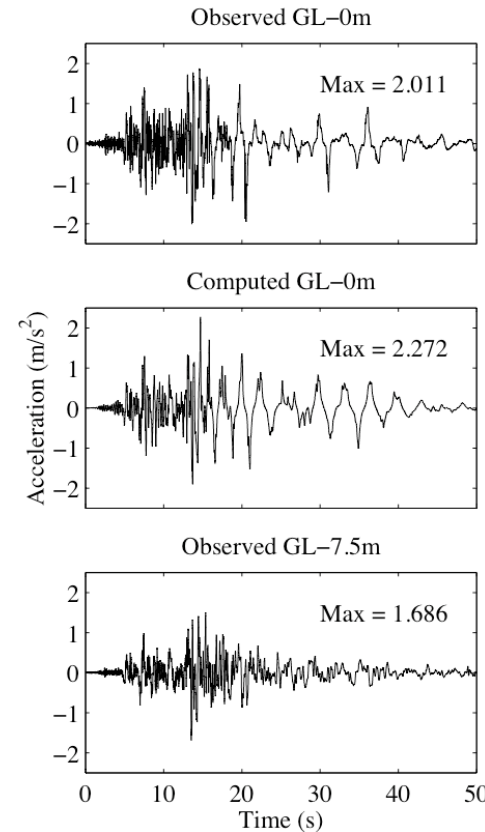
- 1987 Superstition Hills earthquake, M_L 6.6
- Wildlife Refuge site
- Co-located accelerometers and pore pressure transducers (Holzer et al., 1989)
- *In situ* computation of stress-strain time histories and stress path (Zeghal and Elgamal, 1994)

Use of *in-situ* data to calibrate nonlinear rheology

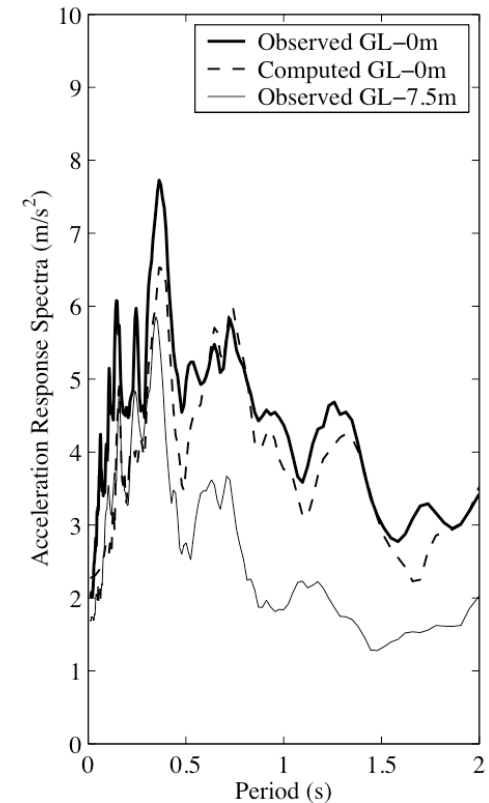
after Bonilla et al. (2005)



Stress computation from deformation data



Waveform modeling



- Model calibration by fitting stress-strain data (and pore pressure)
- Good fit in terms of acceleration and response spectra
- Hint: laboratory data can also be used; always fitting stress-strain data

Empirical evidence of nonlinear site response at a scale of a sedimentary basin

Empirical amplification at sedimentary sites after the Northridge M6.7 earthquake (Field et al., 1997)

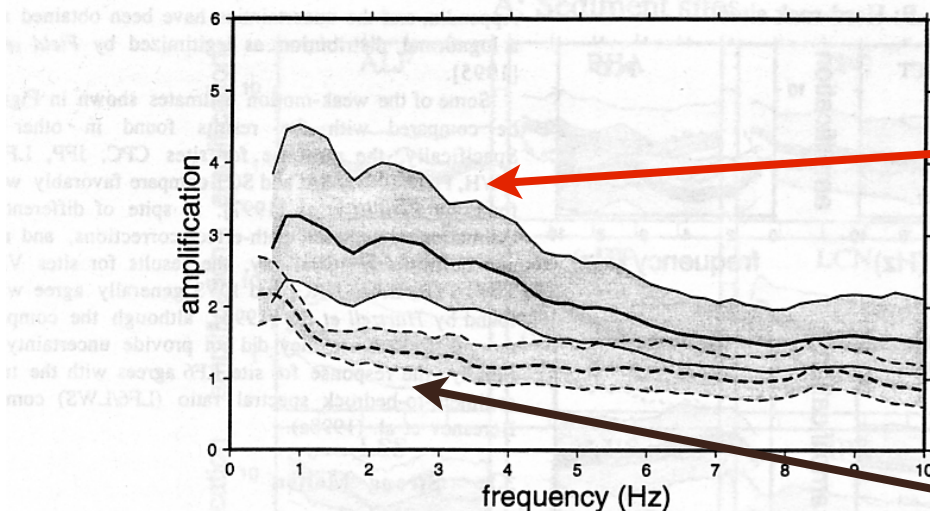
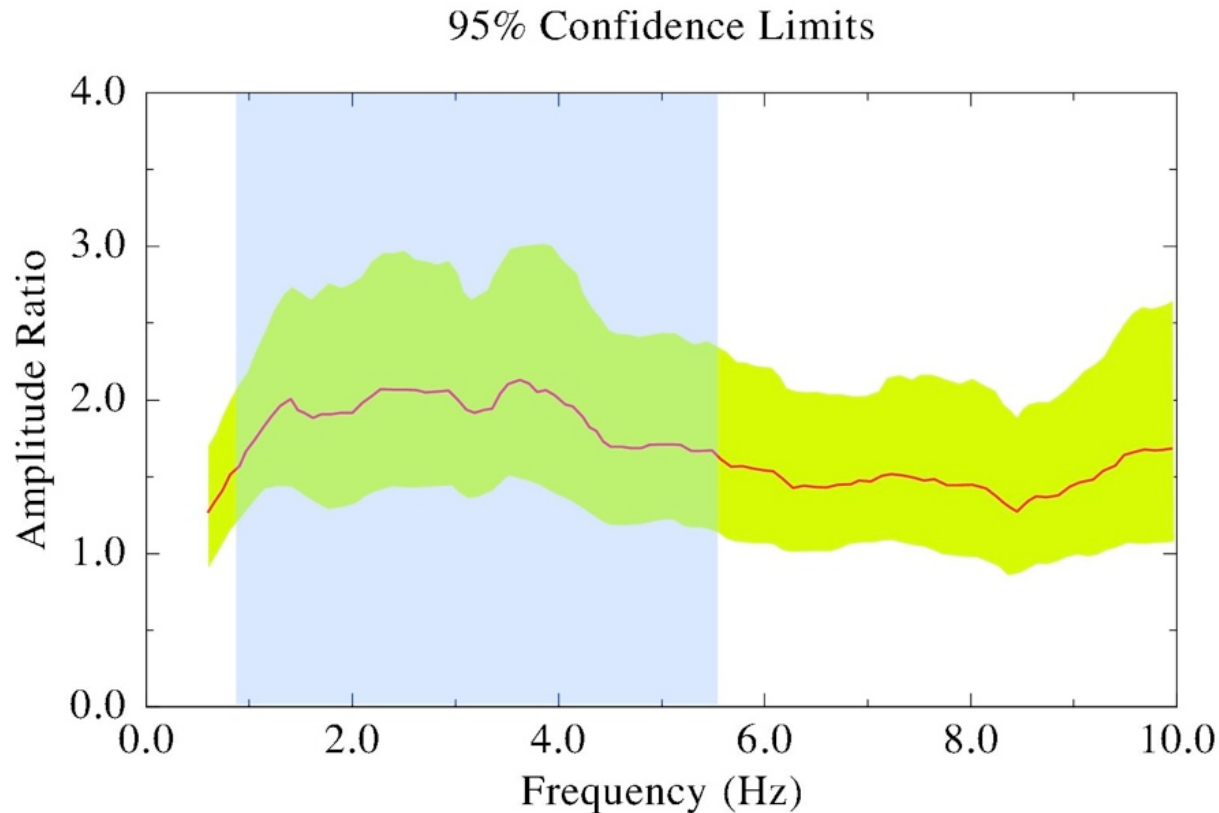


Figure 3. The mean and ± 2 standard deviation of the mean confidence limits for the 15 alluvium site-amplification estimates. The solid lines represent the weak-motion results for the aftershocks, and the dashed lines represent the strong-motion results for the main shock. Reprinted by permission from *Nature* [Field et al., 1997, Copyright 1997, Macmillan Magazines Ltd.].

Aftershocks
(weak-motion)

Mainshock
(strong-motion)

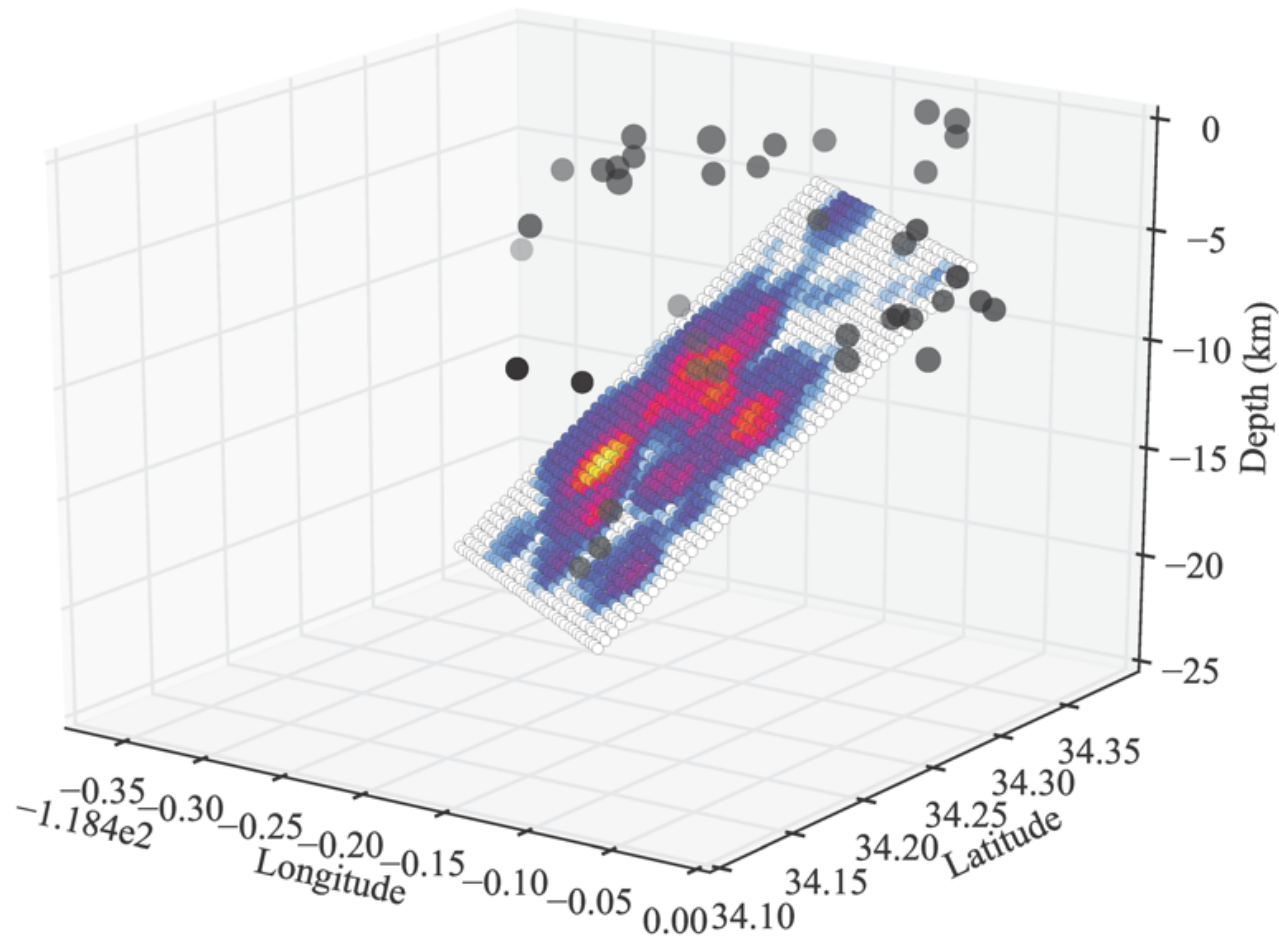
Statistical analysis (Field et al., 1997)



- Weak/Strong motion ratio (if ratio > 1 , nonlinear effects occurred)
- 95% confidence limits defining the acceptance criterion
- Strong motion is “significantly” deamplified between 0.8 and 5.5 Hz

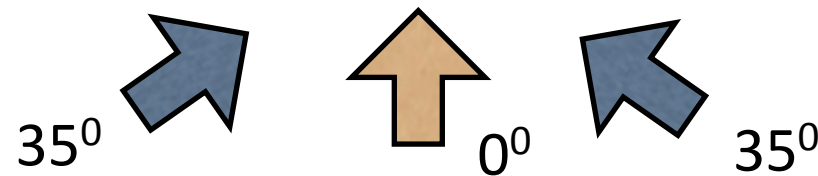
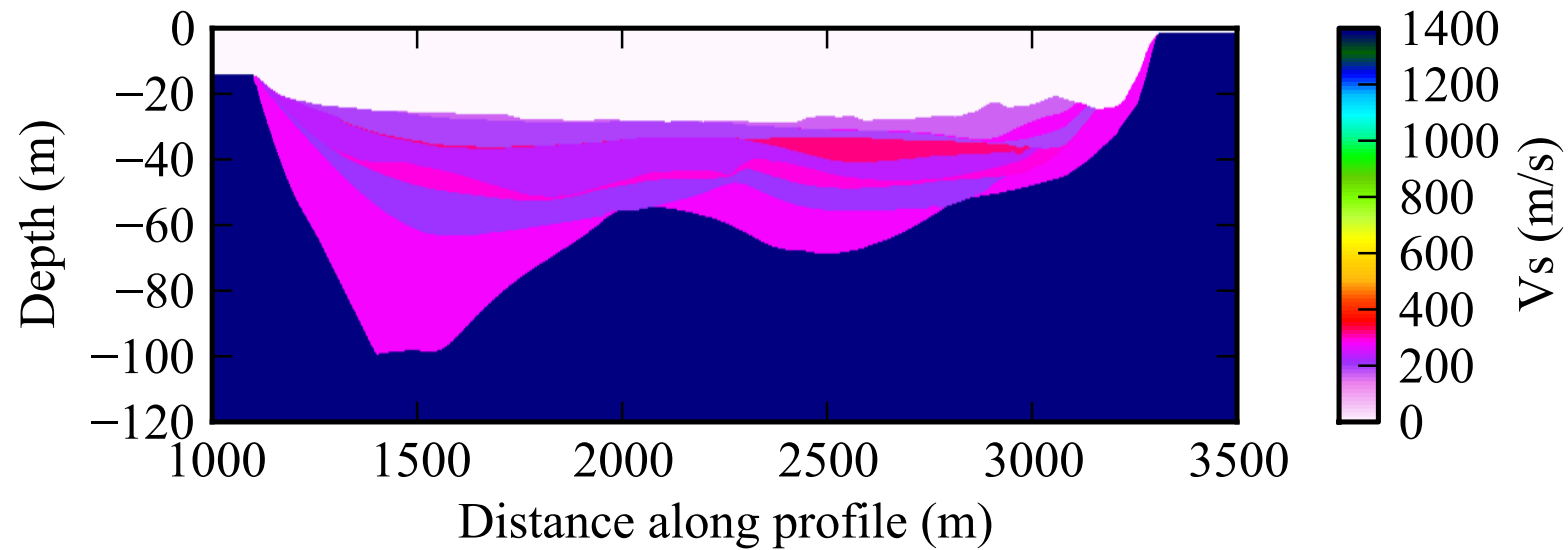
Why is the observed deamplification located at intermediate frequencies?

1994 Northridge aftershocks



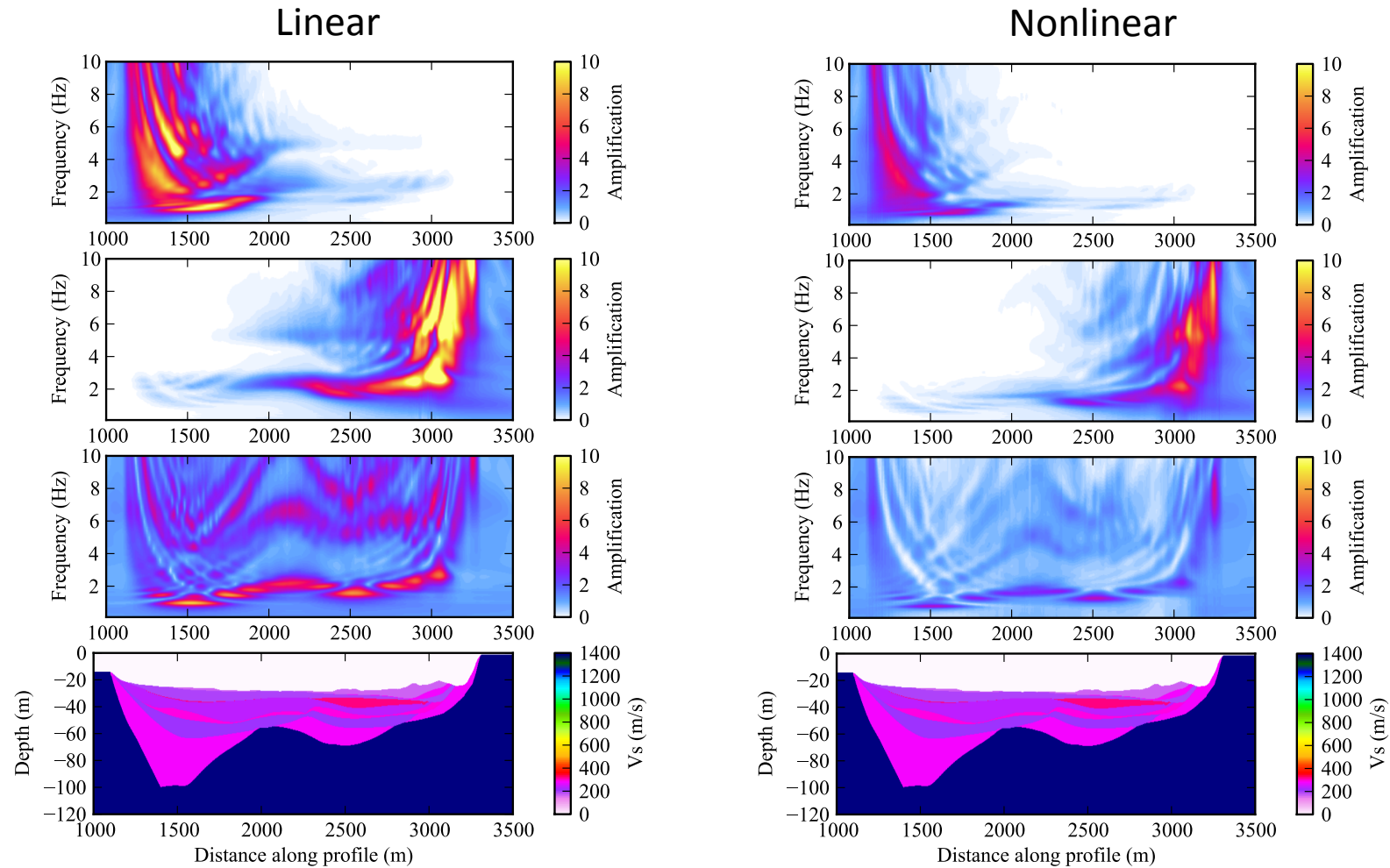
Use of aftershock data implies different angles of incidence of the incoming wavefield

2D response of a smaller basin



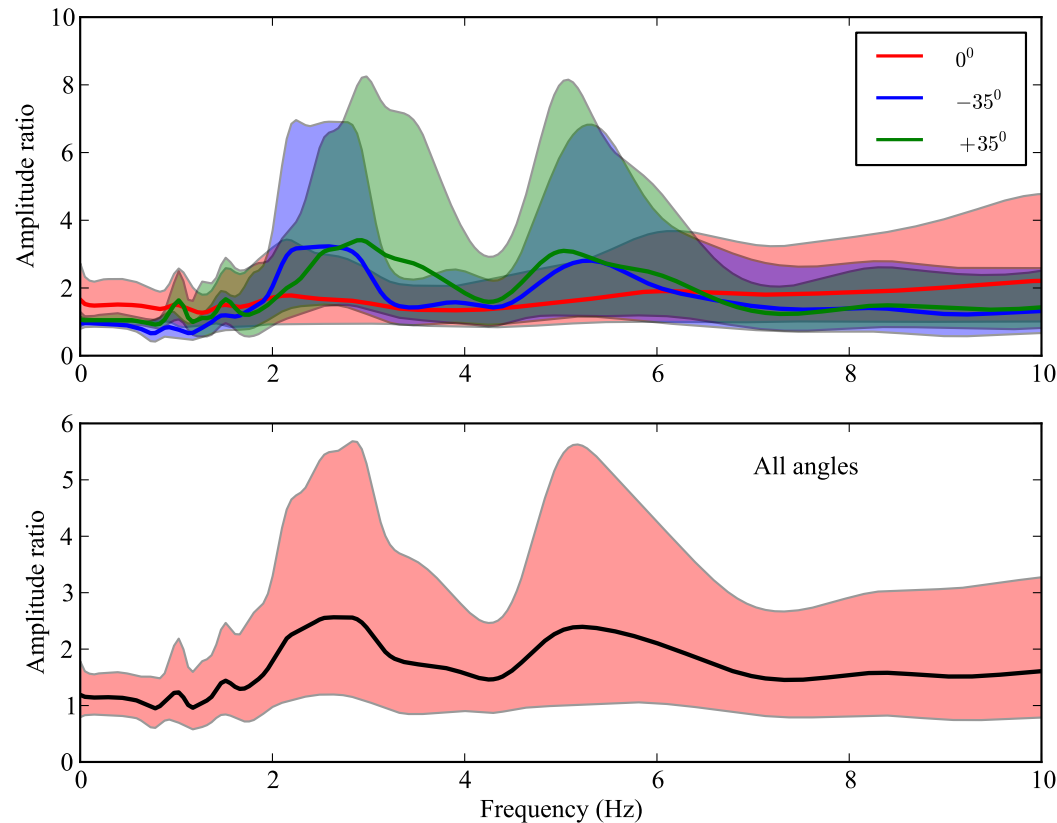
- Available 3D model for the area of Nice, France (CETE Mediterranean)
- Study of the effect of angle of incidence
- Linear and nonlinear basin response up to 10 Hz

Basin response (transfer function) - PGA=0.1g



- “Traditional” nonlinear response mainly observed for vertical incidence
- Vertical incidence underestimates the amplification at the basin edges (linear and nonlinear results)
- Yet, broadband amplification is observed at basin edges for inclined wavefield (linear and nonlinear results)

Statistical analysis as Field et al. (1997)

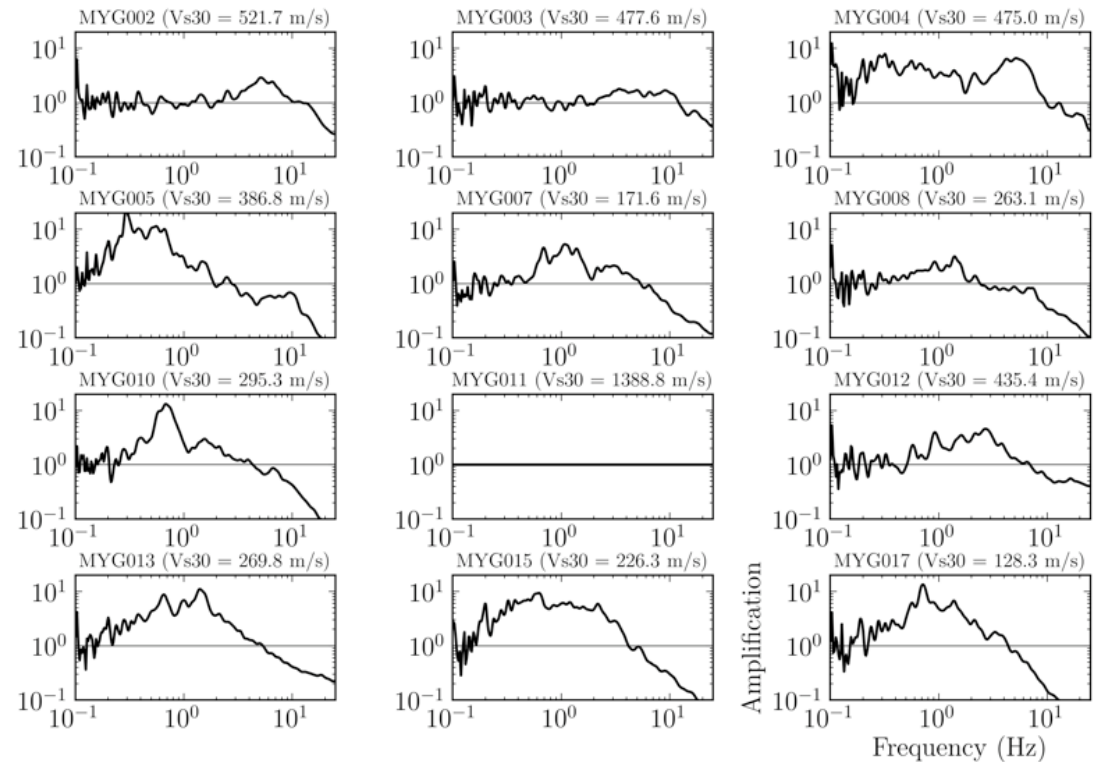
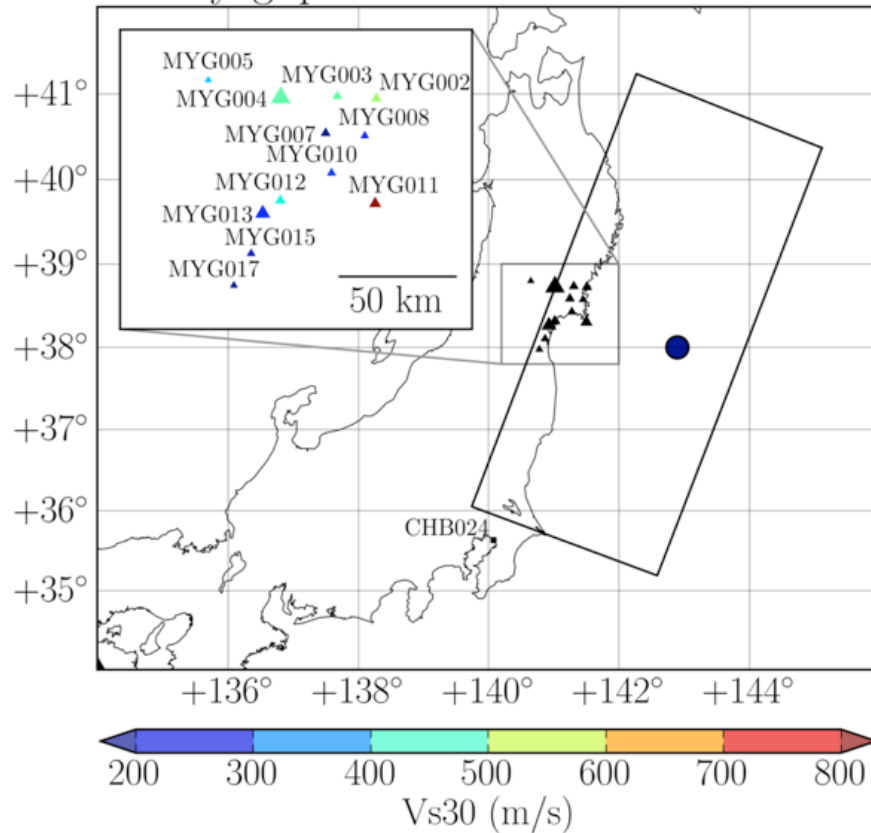


- Computation of mean linear/nonlinear response ratio along the basin profile
- Vertical incidence shows an almost constant linear/nonlinear ratio
- Inclined incidences show stronger nonlinear effects (ratio ~ 3) at 2.5 and 5.5 Hz
- Average linear/nonlinear ratio (including all angles) shows stronger nonlinear effects at these intermediate frequencies than at high frequencies

Northridge nonlinear signature maybe related to angle of incidence
(use of aftershock data only)

The M_w 9, 2011 Tohoku earthquake

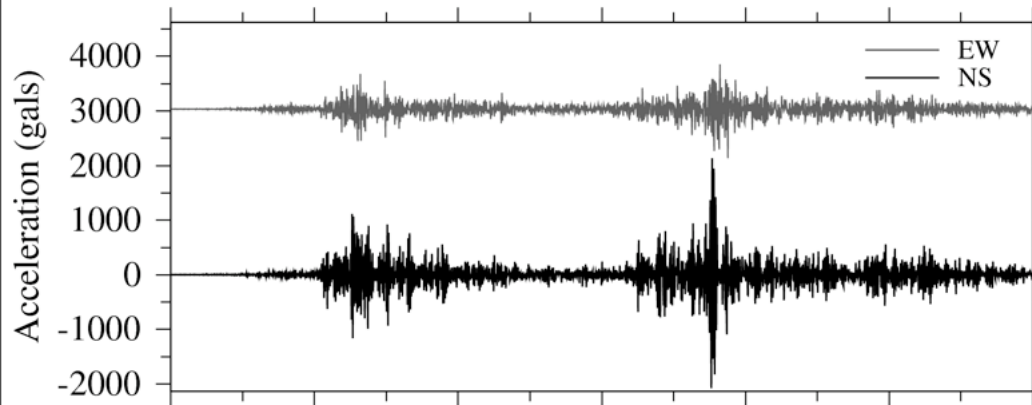
Miyagi prefecture - K-NET stations



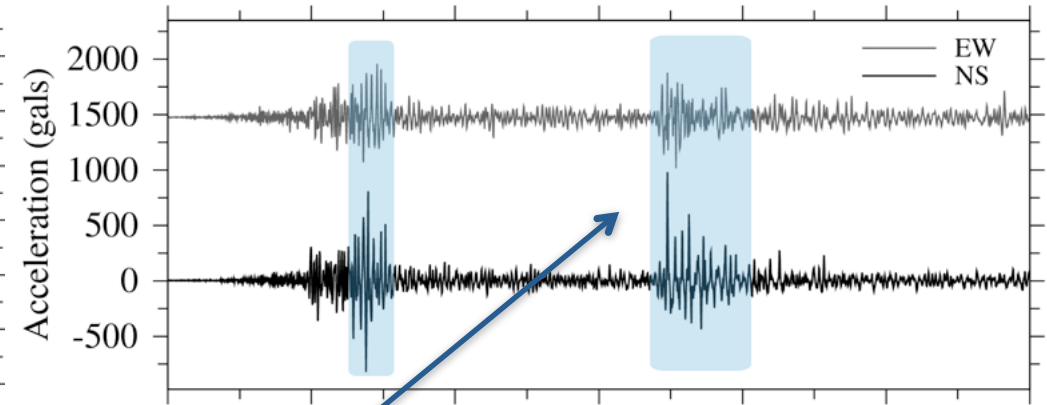
- Traditional spectral ratios w.r.t. MYG011 ($Vs_{30} \sim 1400$ m/s)
- Deamplification at frequencies > 5 -8 Hz ($Vs_{30} < 400$ m/s)
- Strong amplification at low frequencies
- Difficulty to separate source, path and site effects

Time-frequency analysis

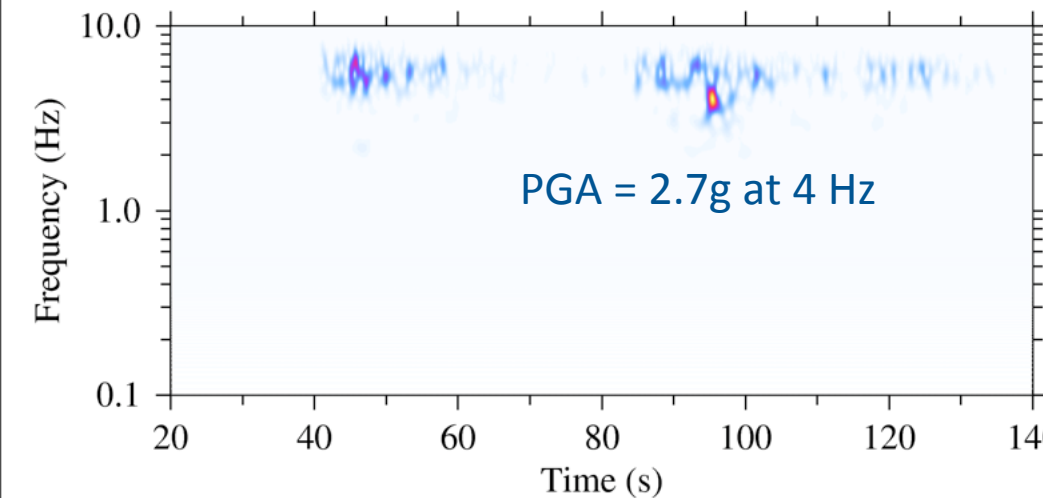
MYG004 ($V_{s30} = 475.0$ m/s)



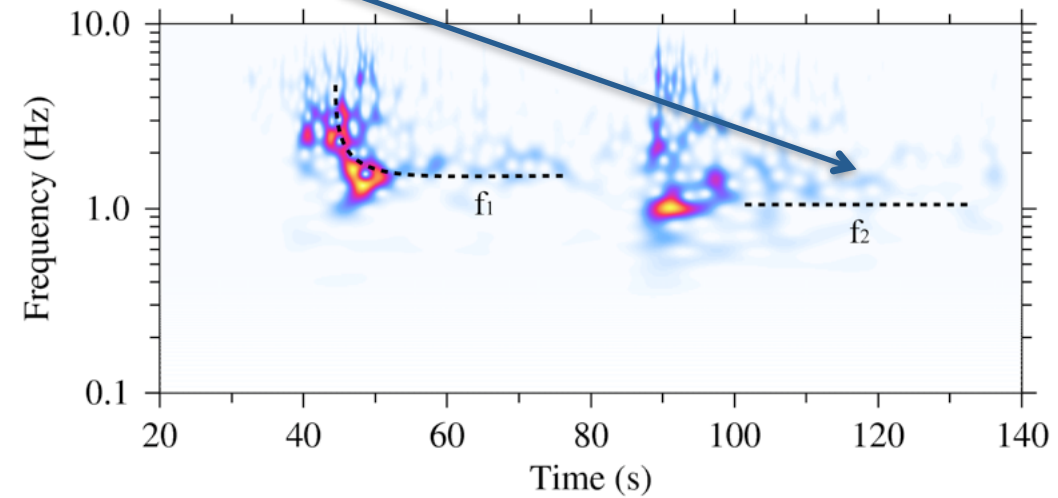
MYG013 ($V_{s30} = 269.8$ m/s)



ESD in horizontal plane (S-transform)



Cyclic mobility



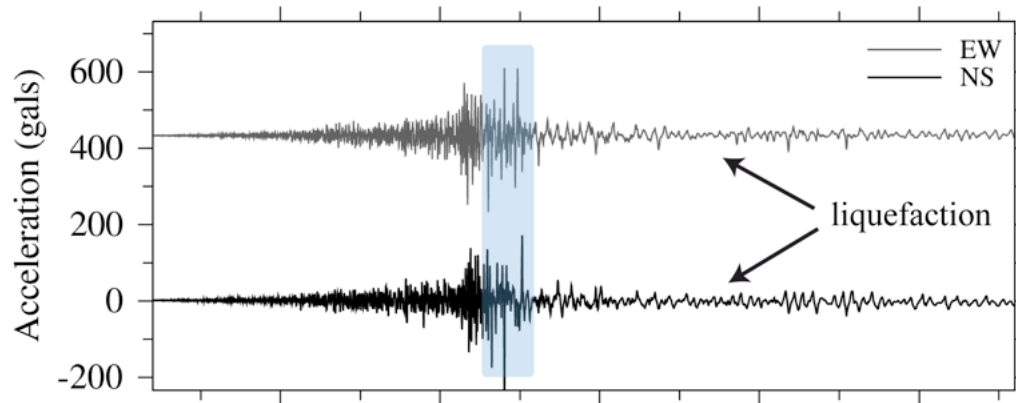
MYG004: no energy above 8 Hz since the beginning of strong motion

MYG013: empirical evidence of cyclic mobility

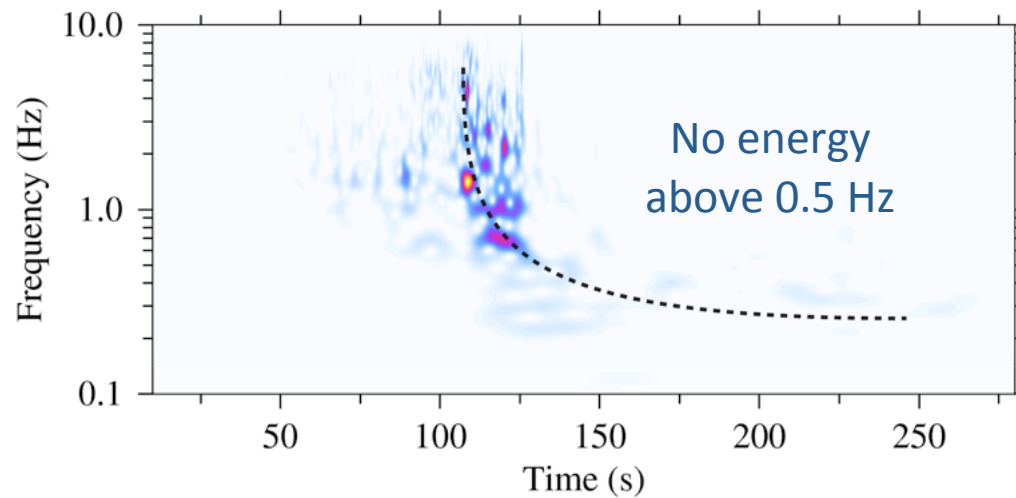
strong shift of energy to lower frequencies at each event's rupture

Chiba - near Tokyo

CHB024 ($V_{s30} = 248.5$ m/s)



ESD in horizontal plane (S-transform)



K-NET稲毛

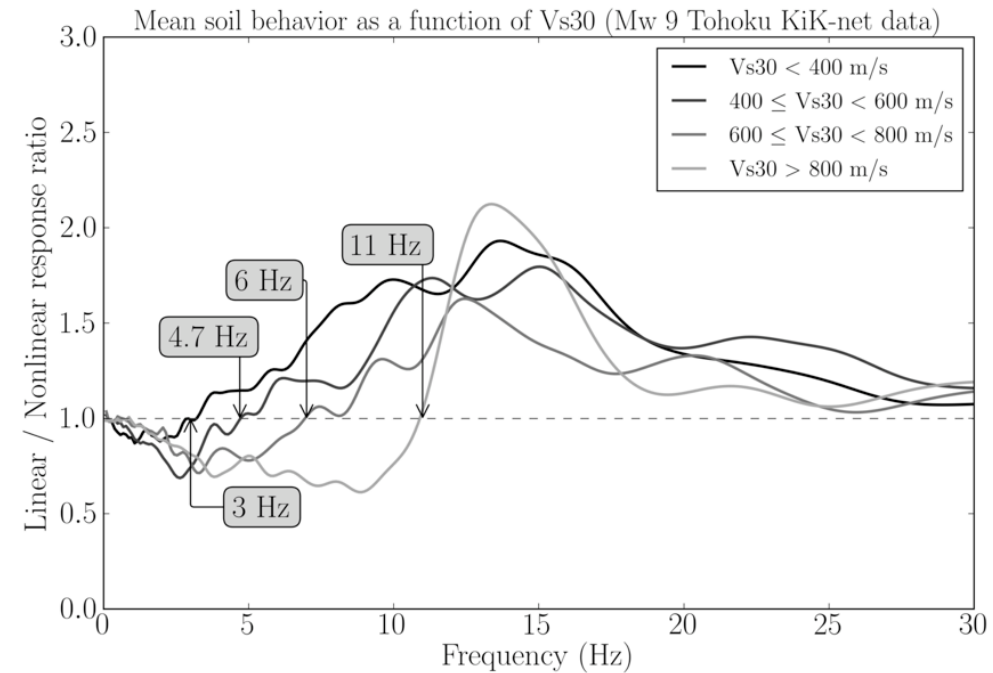
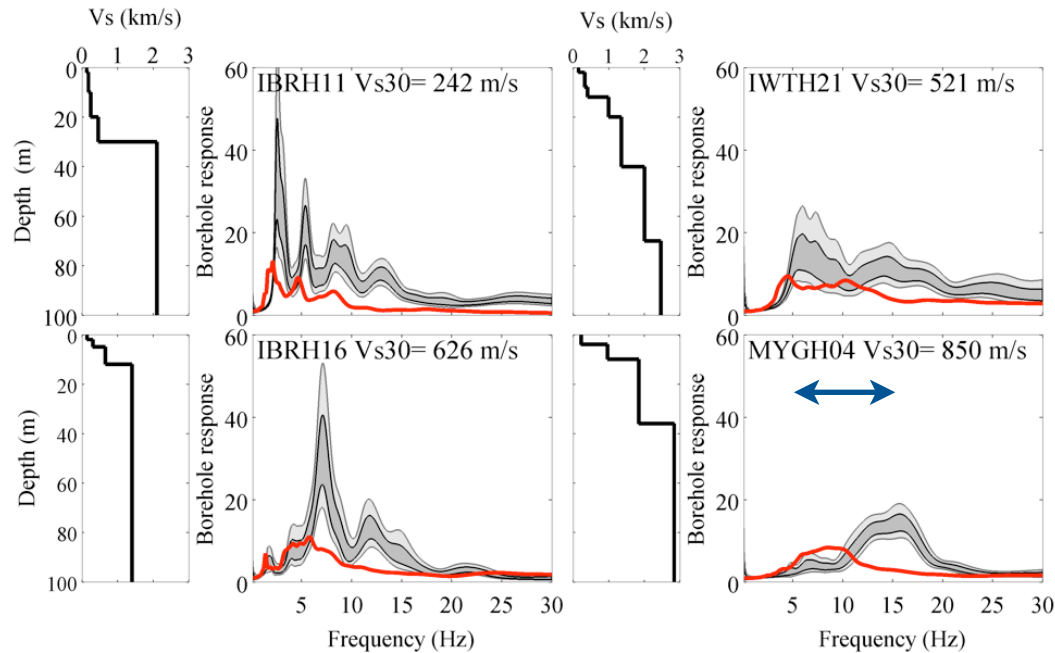
観測点周辺では多くの噴砂が見られ、電柱が傾くなどの被害もあった



Sekiguchi and Nakai, 2011

Cyclic mobility + liquefaction

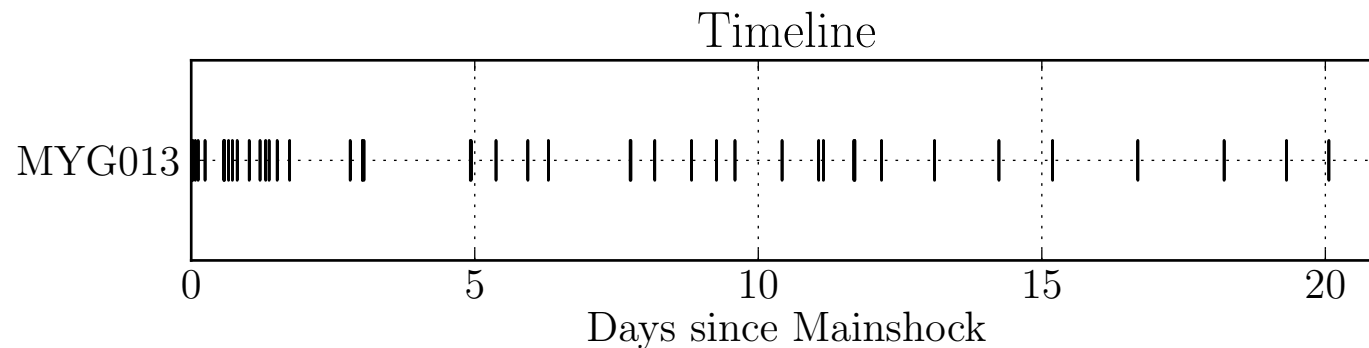
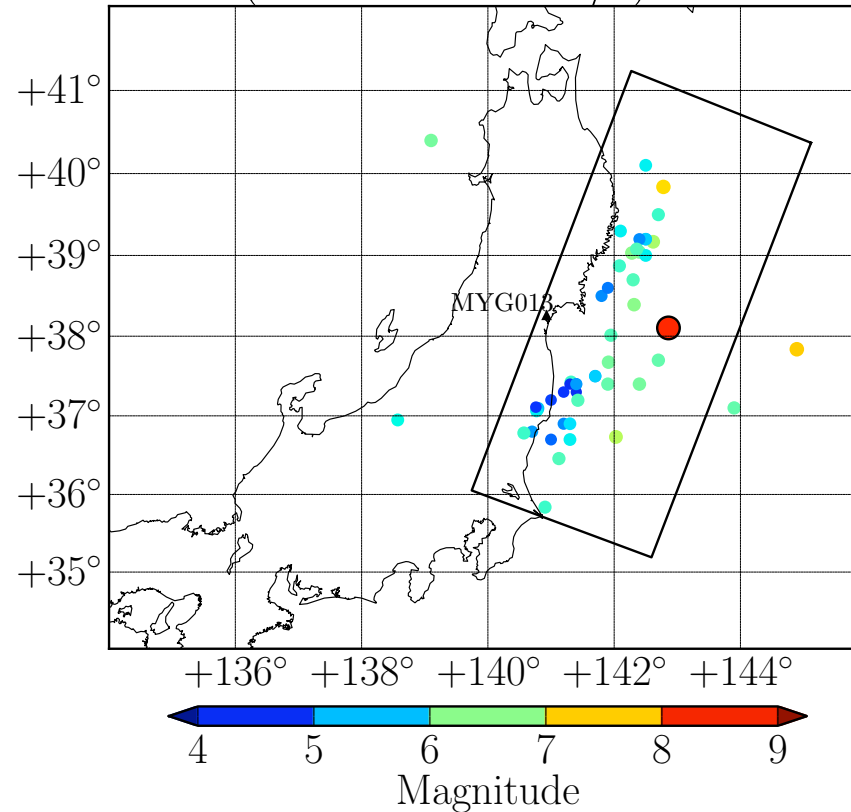
Nonlinear soil response of KiK-net stations and correlation with Vs30



- Deamplification begins at higher frequencies as Vs30 increases
- Shift of “predominant” frequency toward low frequencies
- Rock sites (Vs30 > 800 m/s) also show nonlinear effects at $f > 10$ Hz due to thin shallow layers having Vs30 ~ 400 m/s

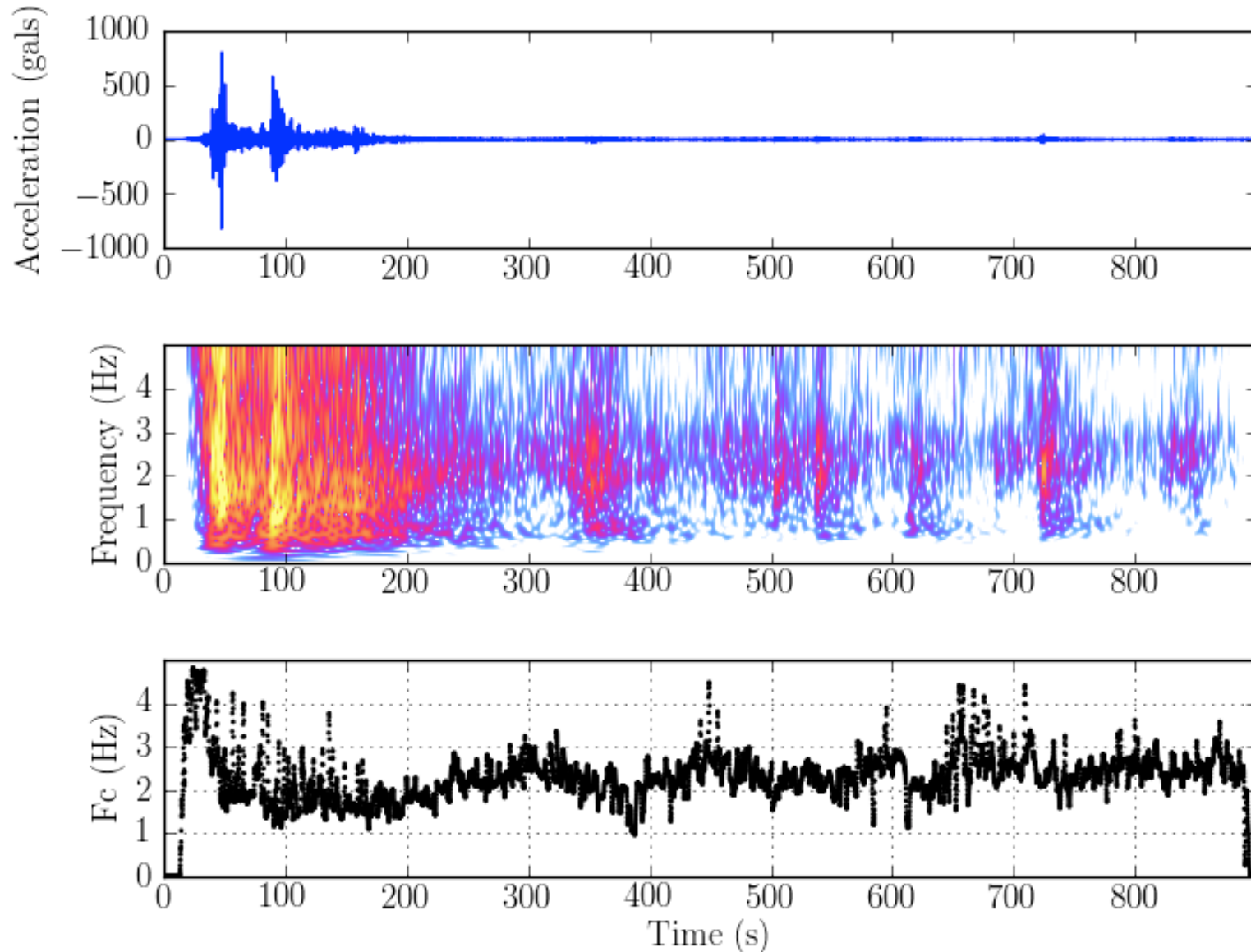
What about soil strength healing? (MYG013)

MYG013 ($V_{s30} = 269.8$ m/s) - March 2011



Compiling aftershock data in the first days (20 days)

What about soil strength healing? (MYG013)



- Example of first 900 seconds
- S-transform of data and central frequency evolution
- F_c decreases for mainshock but comes back after 400 s

Lessons learnt from these events

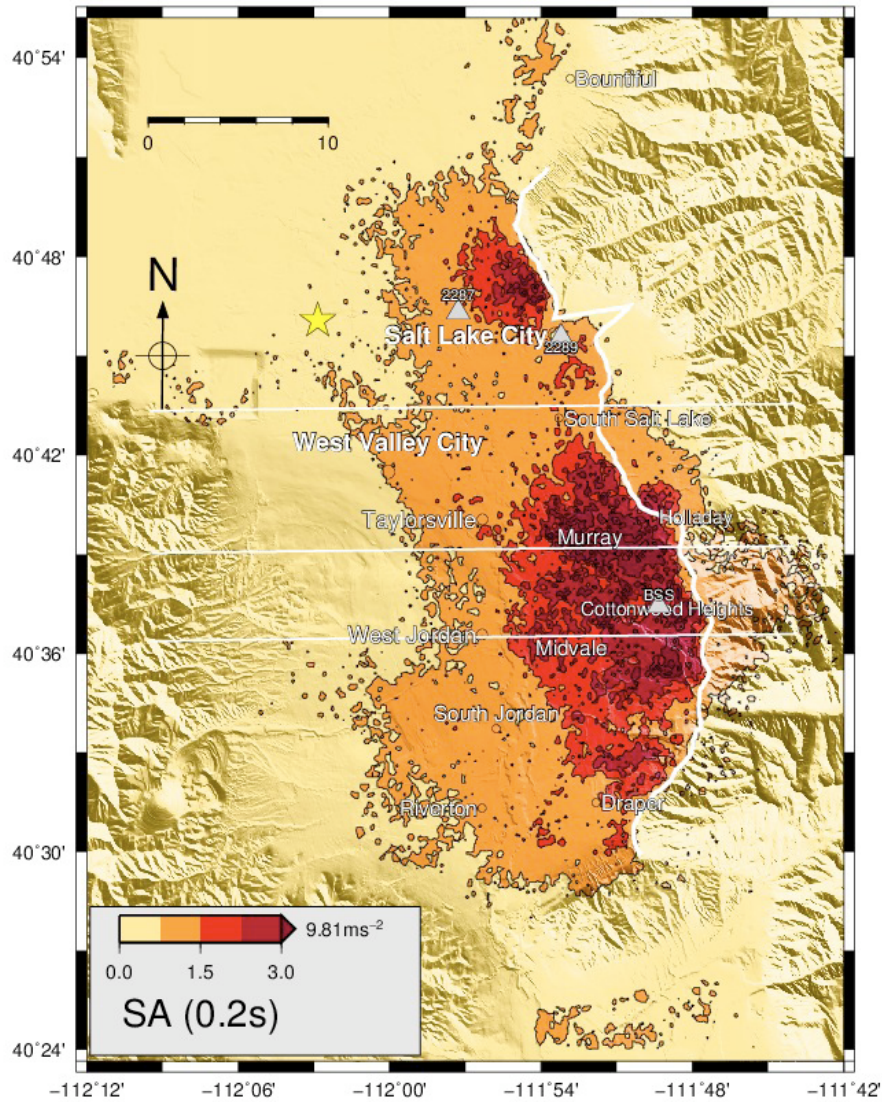
- Densification of surface and borehole arrays including pore pressure transducers to study *in situ* site effects (direct computation of site response and inversion of nonlinear material properties)
- Borehole arrays are very useful to quantify linear and nonlinear effects and possible correlation with Vs30
- It seems that nonlinear effects are shallow (predominant frequency is more affected than the fundamental one)
- PGA only is not enough to discriminate nonlinear effects (cumulative effects of nonlinear behavior)
- The 2011 Tohoku earthquake shows the need to take into account source, path, and nonlinear site response (large scale studies) to better assess the seismic hazard

Perspectives

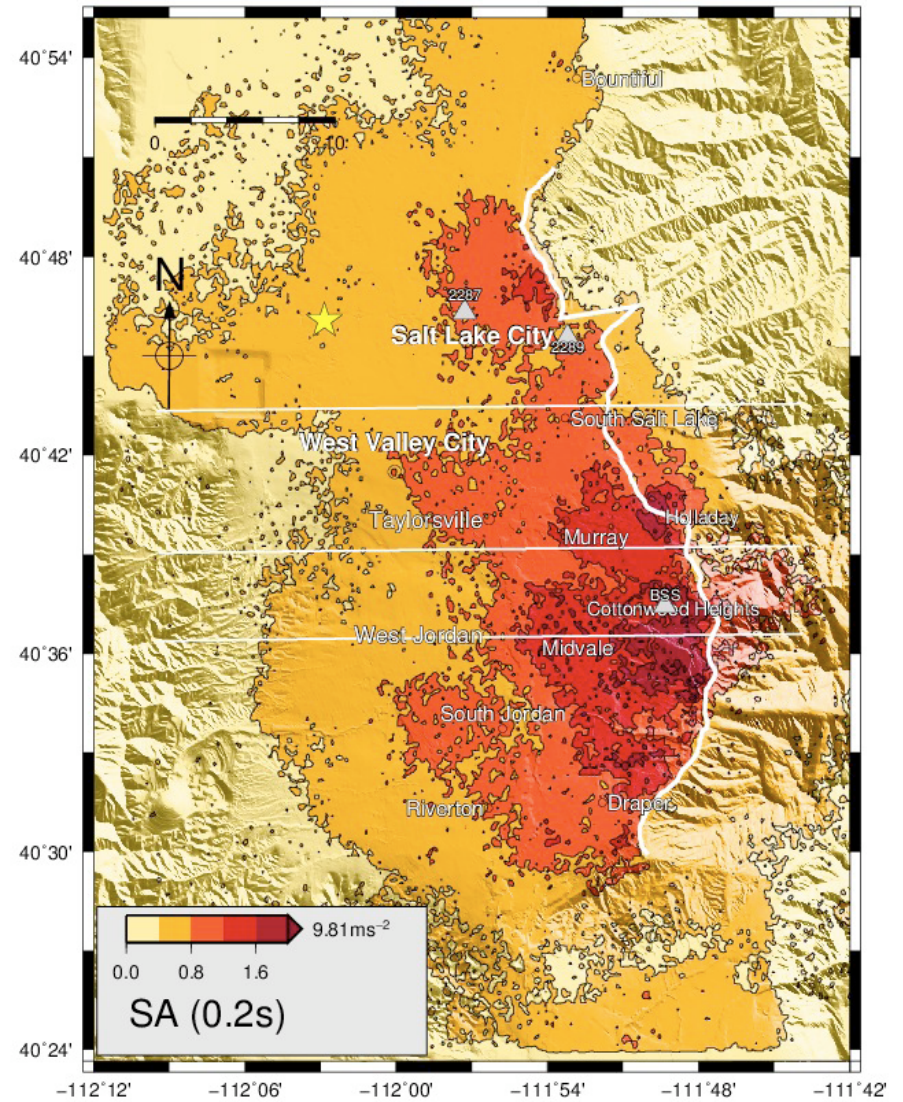
- We need geological/geotechnical/geophysical characterization (statistical analysis of spatial variability of material properties)
- Seismology and earthquake engineering communities should work together (i.e. development of simple but robust nonlinear soil rheologies (GMPE's), soil-structure interaction studies having realistic input motions, etc.)
- Northridge and Tohoku earthquakes, among other events (i.e. NGA data), provide empirical constraints to modelers
- Can we explain nonlinear effects with 1D or 2D models only? Do we need to go to 3D modeling?
- There is a need to quantify the uncertainty of numerical predictions given the soil and ground motion variability

What is the future?

Linear (3D)



Nonlinear (1D)



Earthquake scenario for Utah basin
broadband simulations up to 10 Hz

Roten et al. (2011)